FUNCTIONAL SERVICING BRIEF

ROYAL ROSE COURT

CITY OF OWEN SOUND GREY COUNTY

PREPARED FOR:

FC ENTERTAINMENT & HOSPITALITY INC.

PREPARED BY:

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APRIL 2023

CFCA FILE NO. 1733-6596

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Revision Number	Date	Comments
Rev.0	April 2023	1 st Submission (OPA/ZBA)

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1.0 INTRODUCTION

C.F. Crozier & Associates Inc. was retained by F.C. Entertainment & Hospitality Inc. to prepare a Functional Servicing Brief in support of the Official Plan Amendment and Zoning By-law Amendment Applications for the proposed site reuse located at 1235 & 1259 3rd Avenue East in the City of Owen Sound, Grey County. For the purposes of this report, the subject property which is subject to the applications will be referred to as the "Site". The location of the site is reflected on the development Site Location Plan included as **Figure 1**.

This Functional Servicing Brief provides information about the existing water and sanitary servicing and stormwater management (SWM) systems within the local area and requirements for future reuse of the Site.

External documents/plans were reviewed over the course of completing this engineering report. As such, the servicing and design considerations contained herein are assisted by the following:

- "Site Plan" ERS Architects (December 2022)
- "Occupancy & Circulation Plan" ERS Architects (December 2022)
- "Owen Sound WaterCAD Model Update Rev.1" City of Owen Sound (April 2022)
- "Owen Sound Sanitary Model" City of Owen Sound (March 2022)
- "Owen Sound Jail Heritage Impact Assessment" Taylor Hazell Architects (January 2020)
- "East Owen Sounds Master Servicing Study" R.J. Burnside & Associates Limited (December 2007)

2.0 SITE DESCRIPTION & BACKGROUND

The Site covers an area of approximately 0.65 ha and currently consists of the historic courthouse, governor's residence, prison, and prison yard. The site is bounded by residential properties to the north, 4th Avenue East to the east, Grey Road 15 to the west, and a fire station to the south.

The existing buildings on the Site are proposed to be re-purposed into an event venue, supplemented by restaurants, a shared office space, and an interactive heritage component. Additional surface parking is also proposed on the Site Plan (ERS Architects, December 2022), which has been included as **Figure 2** in this report.

3.0 WATER SUPPLY

Potable water for the Site will be supplied by the City of Owen Sound municipal water distribution system.

3.1 Existing Potable Water Supply Infrastructure

3.1.1 <u>City of Owen Sound Water Treatment Plant</u>

Domestic water is provided through the Owen Sound Water Treatment Plant (WTP), located on the eastern shore on 3rd Avenue East in Owen Sound. Source water is drawn from the Georgian Bay and treated to the Safe Drinking Water Act (SDWA) standards, and the plant has a rated capacity of 27,276 m³/day. The potable water demand is anticipated to increase from the current maximum

daily demand of 21,179 m³/day, to 24,797 m³/day in 2026 and to 40,925 m³/day at the Ultimate Build-out. Based on this analysis from the Master Servicing Report, and from discussions with the City, the capacity of the WTP is adequate to service the proposed reuse of the Site.

3.1.2 <u>Water Distribution System</u>

The existing water distribution infrastructure at or near the Site includes the following:

- 150 mm diameter watermain on 3rd Avenue East.
- 200 mm diameter watermain on 4th Avenue East.

An existing potable water connection currently services the Site, although due to the age of the infrastructure, no service record sheets exist for the connection to confirm size or material. Refer to **Appendix A** for as-recorded drawings showing the existing potable water distribution infrastructure.

3.2 Proposed Water Servicing Strategy

Water servicing for the Site will be supplied by way of the existing 150 mm diameter watermain on 3rd Avenue East. Since the size, material and age of the existing potable water service connection to the building is unknown, a subsurface utility investigation is recommended during detailed design to confirm the condition of the existing service. If the service is in poor condition, or insufficiently sized, a new service lateral will be installed and the existing service will be decommissioned.

3.3 Water Demand

To estimate the existing and proposed water demands for future development of the Site, the Ontario Building Code (OBC), City of Owen Sound Engineering Standards (2016), and the Ministry of the Environment, Conservation and Parks (MECP) Design Guidelines for Drinking-Water Systems (2008) were consulted to determine the average, maximum day and peak hour water demands generated by past and future use of the Site.

3.3.1 Existing Domestic Water Demand

The Heritage Impact Assessment (Taylor Hazell Architects, January 2020) provides some detail regarding the historical use of the Owen Sound Jail, which can be used to estimate the existing water demand. The report notes that the jail housed approximately 40 prisoners at the same time the Governor's Residence was added.

Since the Owen Sound Design Standards do not provide a daily consumption rate that can be applied to an institutional use, such as a prison, the Ontario Building Code (OBC) was used to estimate the average daily demand based on a similar land use. In this case, it was assumed that the most comparable land use in Table 8.2.1.3.B of the OBC was a Long-Term Care Home. Per the OBC, the average daily demand for a Long-Term Care Home is 450 L/bed/day.

Water demands based on the existing usage of the Site were estimated using the following design criteria:

- Average Flow Rate Long-Term Care Home (per OBC)
- Average Flow Rate Residential Dwelling (per OBC)
- 450 L/bed/day 1100 L/dwelling/day 9.03/13.59
- Max Day/Peak Hour Factors (per MECP Standards)

It is estimated that existing water demands for the Site are as follows:

- Average Day 0.22 L/sec
- Max Day 2.00 L/sec
- Peak Hour 3.00 L/sec

Refer to **Appendix B** for detailed calculations.

3.3.2 Proposed Domestic Water Demand

As noted in Section 2.0 the existing building is to be maintained on site and converted into an event venue, supplemented by restaurants and a shared office space. Based on the Architectural Drawings by ERS Architects in December 2022, the following uses are proposed within the building. Refer to the Occupancy Plan in **Appendix C**.

- Event Venue
- Office Space
- Restaurant
- Speak Easy / Lounge

In order to calculate the proposed water demand, it was assumed that the building would not have all of the above uses operating at the same time; therefore, the following scenarios were established:

- Scenario #1 Event Venue Only
 - Assumes that all other uses are closed during an event.
- Scenario #2 Office Space and Restaurant
 - Assumed to be the typical use during a weekday.
- Scenario #3 Restaurant and Speak Easy/Restaurant
 - Assumed to be the typical use during the evenings.
- Scenario #4 2 Events and Speak Easy/Restaurant
 - Assumed to be the maximum demand scenario possible.

As noted in Section 3.3.1 the Owen Sound Design Standards do not provide a daily consumption rate that can be applied to certain uses, such as an event venue. Therefore, the Ontario Building Code (OBC) was used to estimate the average daily demand based on a similar land use. In this case, it was assumed that the most comparable land use to an event venue in Table 8.2.1.3.B of the OBC was an Assembly Hall with Food Service Provided. Per the OBC, the average daily demand for an Assembly Hall with Food Service is 36 L/seat/day.

Water demands based on the proposed usage of the Site was determined using the following design criteria:

- Average Flow Rate Assembly Hall with Food Service (per OBC) 36 L/seat/day
- Average Flow Rate Office Building (per OBC)
- Average Flow Rate Restaurant (per OBC)
- Average Flow Rate Bar and Cocktail Lounge (per OBC)
- Max Day/Peak Hour Factors (per MECP Standards)

Using the criteria above, the average day water demand was calculated for each scenario. The results are presented in **Table 1** below.

75 L/9.3m²/day

125 L/seat/day

125 L/seat/day

3.60/5.40

	Scenario #1	Scenario #2	Scenario #3	Scenario #4
Average Daily Flow Rate (L/s)	0.10	0.27	0.43	0.58

Table 1: Average Daily Demand Flows

To be conservative, the highest average daily demand flow rate (Scenario #4) has been used to estimate the future water demands for the Site, which are as follows:

- Average Day 0.58 L/sec
- Max Day 2.09 L/sec
- Peak Hour 3.14 L/sec

Refer to **Appendix B** for detailed calculations.

3.3.3 <u>Fire Flows</u>

Fire flows required to service the site were determined to be 183 L/s per the Fire Underwriter's Survey. The total design flow (peak hour + fire flow) for the Site is 186.14 L/s.

At the time this report has been prepared, a hydrant flow test has not been completed. However, upon review of the East Owen Sound Master Servicing Study (R.J. Burnside, 2007), the Site is located in between two Junctions in the City's water model (J-254 and J-256). Based on the model outputs, the fire flows between the two nodes ranges from 144.23 L/s to 228.31 L/s. Refer to the excerpts from the East Owen Sound Master Servicing Study in **Appendix B** for details.

A hydrant flow test will be completed during detailed design to confirm the available fire flows.

4.0 SANITARY SERVICING

4.1 Existing Sanitary Sewer Infrastructure

4.1.1 <u>City of Owen Sound Wastewater Treatment Plant</u>

The City of Owen Sound Wastewater Treatment Plant (WWTP) is located at 2050 3rd Avenue East and discharges into the Georgian Bay. It was constructed in 1962 and has undergone multiple upgrades. It currently has a rated capacity of 24,545 m³/day and a peak daily flow rate of 65,000 m³/day. Based on the analysis from the East Owen Sound Master Servicing Study and the City's Sanitary model, along with discussions with the City, it is determined that the capacity of the existing WWTP is adequate.

4.1.2 <u>Sanitary Sewer System</u>

The existing sanitary sewer infrastructure at or near the Site consists of 300 mm diameter trunk sanitary sewer on 3rd Avenue East. There also exists sanitary sewer infrastructure on 4th Avenue East; however, as-recorded drawings were not available when our office requested them from The City.

An existing sanitary sewer connection currently services the Site, although due to the age of the infrastructure, no service record sheets exist for the connection to confirm size or material.

Refer to Appendix A for as-recorded drawings showing the existing sanitary sewer infrastructure.

4.2 Proposed Sanitary Servicing Strategy

Sanitary servicing for the Site will be provided through connection to the existing City of Owen Sound sanitary sewer connection network, with flows from the Site ultimately receiving treatment within the Owen Sound Wastewater Treatment Plant.

Since the size, material and age of the existing sanitary service connection to the building is unknown, a subsurface utility investigation is recommended during detailed design to confirm the condition of the existing service. If the service is in poor condition, or insufficiently sized, a new service lateral will be installed on 3rd Avenue East and the existing service will be decommissioned.

4.3 Sanitary Demand

The City of Owen Sound Engineering Standards (2016), MECP Design Guidelines for Sewage works (2008), and the Ontario Building Code were used to determine the estimated sewage demands for the existing and future conditions of the Site.

4.3.1 Existing Sanitary Demand

As noted in Section 3.3.1, the Owen Sound Design Standards do not provide a daily consumption rate that can be applied to an institutional use, such as a prison. Therefore, the Ontario Building Code (OBC) was used to estimate the average daily demand based on a similar land use. In this case, it was assumed that the most comparable land use in Table 8.2.1.3.B of the OBC was a Long-Term Care Home.

Sanitary flows for the existing usage of the Site were determined using the following design criteria:

- Average Flow Rate Long-Term Care Home (per OBC)
- Average Flow Rate Residential Dwelling (per OBC)
- Residential Peaking Factor
- Institutional Peaking Factor
- Infiltration (City of Owen Sound standards)

Based on these values it is estimated that peak sanitary flow from the existing conditions of the Site is 1.01 L/sec. Refer to the calculations in **Appendix D**.

4.3.2 Proposed Sanitary Demand

In order to calculate the proposed sanitary demand, the same four scenarios that were used to calculate the water demand in Section 3.3.2 were evaluated:

• Scenario #1 – Event Venue Only

- Assumes that all other uses are closed during an event.
- Scenario #2 Office Space and Restaurant
 - Assumed to be the typical use during a weekday.
- Scenario #3 Restaurant and Speak Easy/Restaurant
 - Assumed to be the typical use during the evenings.

• Scenario #4 – 2 Events and Speak Easy/Restaurant

• Assumed to be the maximum demand scenario possible.

C.F. Crozier & Associates Inc. Project No. 1733-6596 450 L/bed/day 1100 L/dwelling/day 4.0 (Harmon Formula) 4.0 (Harmon Formula) 0.20 L/s/ha Similar to what was done in Section 3.3.2, the Ontario Building Code (OBC) was used to estimate the average daily demand based on a similar land use. In this case, it was assumed that the most comparable land use to an event venue in Table 8.2.1.3.B of the OBC was an Assembly Hall with Food Service Provided. Per the OBC, the average daily demand for an Assembly Hall with Food Service is 36 L/seat/day.

Sanitary flows for the future usage of the Site were determined using the following design criteria:

- Average Flow Rate Assembly Hall with Food Service (per OBC)
- Average Flow Rate Office Building (per OBC)
- Average Flow Rate Restaurant (per OBC)
- Average Flow Rate Bar and Cocktail Lounge (per OBC)
- Institutional Peaking Factor
- Infiltration (City of Owen Sound standards)

Based on these values it is estimated that peak sanitary flow from the future conditions of the Site will be 2.40 L/sec. Refer to the calculations in **Appendix D**.

4.3.3 <u>Wastewater PCSWMM Model</u>

A wastewater PCSWMM model was provided by the City on March 3, 2023. This model was used to analyze the sanitary sewers downstream of the site under existing and proposed dry weather flow conditions. The sewers downstream of the site until the 1,000 mm diameter trunk sewer located at the intersection of 13th Street East and 2nd Avenue East were analyzed.

Under existing conditions, the maximum hydraulic grade elevation (HGL) of the manhole immediately downstream of the site (SSMH1368) is 180.99m. The 300 mm diameter sewer fronting the site is at a slope of 0.5% and has a capacity of 68.4 L/s. The results show that the maximum flow in this pipe is 0.14 L/s; therefore, the pipe is 0.2% full under existing conditions. Please refer to **Appendix D** for a sewer profile of existing conditions from Junction SSMH1368 to SSMH1357.

A model was created to demonstrate proposed conditions. A conservative approach was used when updating the site flows to demonstrate proposed conditions. As described in the section above, the sanitary demands for existing and proposed conditions were calculated. The increase in demand (1.39 L/s) was added to the Average Value input in the model at Junction SSMH1366. The proposed model was run and the maximum HGL of Junction SSMH1366 was negligibly increased to 181.01m. The maximum flow of this pipe under proposed conditions is 1.87 L/s, therefore, the pipe is 2.7% full. Please refer to **Appendix D** for a sewer profile of proposed conditions.

Based on the model results, there is sufficient capacity in the existing sanitary sewers to support the proposed reuse without any sewer upgrades.

36 L/seat/day 75 L/9.3m²/day 125 L/seat/day 125 L/seat/day 3.9 (Harmon Formula) 0.20 L/s/ha

5.0 STORMWATER MANAGEMENT AND SITE DRAINAGE

5.1 Stormwater Management Criteria

The management of stormwater and Site drainage for the entire Site must comply with the policies and standards of the various agencies including the City of Owen Sound and the Ministry of Environment, Conservation and Parks (MECP).

The stormwater management criteria for the future development include:

- Water Quantity Control
 - "Post to Pre" control for storms up to and including the 100-year event for any future development;
 - Quantity Control is required for future development; and
- Water Quality Control
 - "Enhanced Protection" given Georgian Bay as the ultimate receivers.

5.2 Existing Drainage Conditions

A topographic survey was completed by Hewett and Milne Ltd. (February 2023), which confirmed that the Site drains east to west toward 3rd Avenue East. Both the minor and major storm flows are conveyed via sheet flow across the site before being conveyed on the 3rd Avenue East ROW to the Georgian Bay. Refer to **Appendix E** to view the topographic survey completed for the site.

5.3 Proposed Drainage Conditions

Per the Town's engineering standards, the proposed drainage system within the development will be required to convey internal runoff to an appropriate outlet and all infrastructure must be sized accordingly. Through discussions with the City, it is understood that the preferred stormwater outlet is the existing storm sewer on 12th Street East, which outlets directly to Georgian Bay. Although there is no existing storm sewer on 3rd Avenue East, there is an existing 375mm diameter storm sewer on 4th Avenue East that conveys runoff to the sewer on 12th Street East.

Internal paved areas within the site will be graded with varying slopes typically ranging from 0.5% - 5% to promote stormwater drainage from the parking area, towards proposed catchbasins and ultimately connecting to the existing storm sewer infrastructure on 4th Avenue East.

The proposed reuse of the Site will not significantly alter the current grading and drainage conditions. Under post-development conditions, the minor storm events will be conveyed through a storm sewer network within the proposed parking lot that will connect to the existing 375 mm diameter storm sewer on 4th Avenue East. The major storm events will be conveyed overland towards 3rd Avenue East.

5.4 Stormwater Quantity Control

Based on the Site Plan, the Site will have an increase in impervious area with the additional parking lot off 4th Avenue East. Stormwater runoff for the minor flows will be directed to 4th Avenue East via storm sewers, and major flows will continue to 3rd Avenue East via overland flow. Post-development peak flows will be controlled to meet the existing pre-development peak flows through use of a stormwater management facility on site, providing the required volume of storage.

Per the City of Owen Sound Engineering Standards Section C - Storm Drainage, the Rational Method was used to calculate the pre- and post-development flow rate produced on site for the 2-year to 100-year storm events. The Intensity Duration Frequency Curves for Owen Sound were used in the calculations.

Runoff coefficients were estimated by measuring the impervious and pervious areas for the site under pre- and post- development conditions. Land Use runoff coefficients from Section 3.5 of the City of Owen Sound Engineering Standards were applied to the impervious and pervious areas. A weighted average was calculated to determine a design runoff coefficient for the site in pre- and post-development which was used to estimate peak flows. Pre- and post-development flows are summarized in **Table 2**. Refer to **Appendix F** for the full calculations.

Pre-Development (m ³ /	Peak Flow Rate	Post-Development Peak Flow Rates (m³/s)			
5 yr 100 yr		5 yr	100 yr		
0.12	0.26	0.17	0.37		

Table 2: Pre and Post Development Peak Flow Rates

The results of the Preliminary Rational Method calculations shown in **Table 2** establish that on-site storage will be required to control post-development flows to pre-development flows because the added impervious area of the proposed parking lot increased peak flows for all storm events. The preliminary calculations determined that approximately 51 m³ of storage volume will be required. **Table 3** below provides an overview of various methods of quantity control and their suitability for the site, which may be implemented during detailed design.

Table 3: Stormwater Management (SWM) Facility Options					
SWM Facility Type	Comments	Consideration (Yes/No)			
Wet Pond	Wet ponds are an effective way to control the flows and are not affected by groundwater and bedrock. This form of quantity control requires a drainage area greater than 5ha to sustain a permanent pool, as per MOE design guidelines. Since the site is less than 5ha it is not recommended as a SWM Facility.	No			
Dry Pond	Similar to wet ponds, dry ponds take up large areas and are recommended for drainage areas greater than 5 ha. Since the site is less than 5ha it is not recommended as a SWM Facility.	No			
Infiltration Basin	Infiltration basins are acceptable for smaller areas, but is not a suitable option due to the limited space available on the site.	No			
Surface Storage	The Municipality's Development Standards allow for surface storage up to a certain depth to reduce peak flow rates to storm sewers. Surface storage will likely be implemented within the proposed parking lot.	Yes			
Rooftop Storage	Since the proposed building is being maintained, it is unlikely that the roof could be altered in order to provide additional storage.	No			
Super Pipes	Storage within the pipes, manholes and catchbasins located within a roadway or parking lot can be utilized in conjunction with an orifice plate placed on an outlet structure downstream to provide storage internally. Although this type of facility likely cannot be used as a stand-alone system, as it does not provide sufficient quality controls, it may be used as part of the stormwater management strategy for the site.	Yes			
Subsurface Storage Tanks	Similar to superpipes, subsurface storage tanks can provide large volumes of storage without reducing the developable area of a site. These types of SWM facilities are typically used in parking lots and may be suitable for this Site.	Yes			
Low Impact Designs (LIDs)	Landscaped areas provide opportunities to include LIDs such as infiltration trenches and soak-away pits into the grading plans to provide additional quantity control. Based on the available soils mapping, it appears that the soils on site may be suitable for LIDs; however, this would need to be confirmed with a geotechnical investigation and hydrogeologic assessment.	Maybe			

5.5 Stormwater Quality Control

It will be necessary to implement stormwater best management practices to address the water quality control requirements of the Municipality and other regulatory agencies. Georgian Bay is the ultimate receiver of drainage from the Site and therefore the development will incorporate measures to provide "enhanced protection" to treat runoff from the site. To provide "enhanced protection" an end-of-pipe control is recommended to treat the runoff from the proposed parking lot before it enters the Municipal storm sewer system.

5.5.1 <u>End-Of-Pipe Controls</u>

Oil/grit separators are typically recommended to treat runoff from the roadways and parking lots, which are the main source of oils and sediment from the vehicles. These structures are typically premanufactured and provide effective removal of oils and total suspended solids. The oil/grit separators are sized to treat minimum 95% annual runoff and a minimum 80% of annual total suspended solids (TSS) removal.

6.0 CONCLUSIONS AND RECOMMENDATIONS

It is concluded that the proposed reuse of the Site can be fully serviced by way of a storm sewer connection to 4th Avenue West, a new sanitary service lateral (if determined to be required by subsurface investigation), and a new water service lateral (if determined to be required by sub-surface investigation). Stormwater quality control can be met via and oil-grit separator.

We trust that this report will be sufficient in supporting the Official Plan and Zoning By-Law Amendment Applications. Should you have any questions or require further information, please do not hesitate to contact the undersigned. Thank you.

Respectfully submitted,

C.F. CROZIER & ASSOCIATES INC.

Nicholas Sproule, EIT Engineering Intern

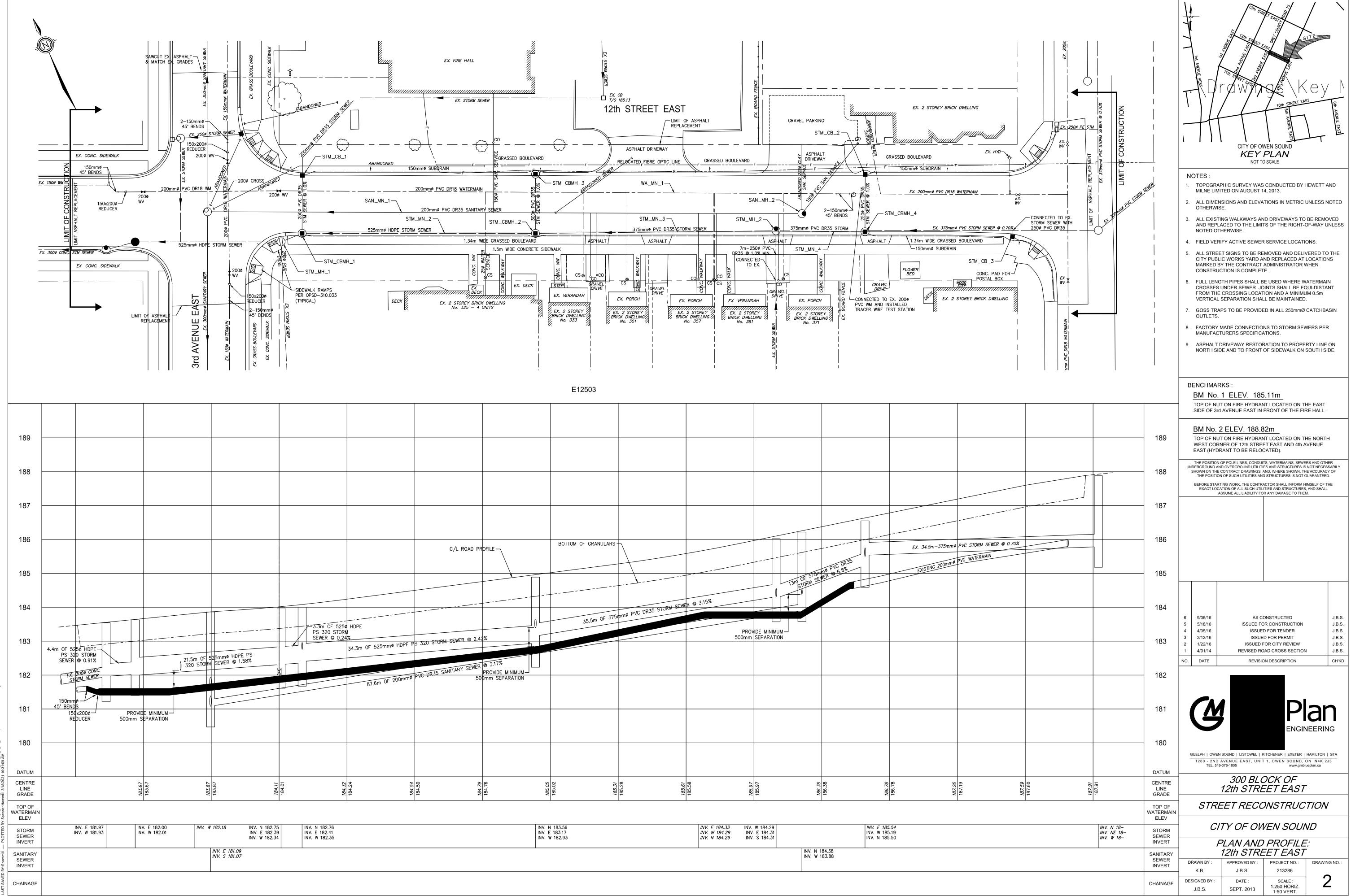
C.F. CROZIER & ASSOCIATES INC.

George Cooper, P.Eng. Project Manager

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As-Recorded Information



anmil/AppDatalLocal\TempTemp1_05-11-17_152612_213286_12th_St_E_As_Built.zip/213286 GP-K As Built.dwg LAYOUT:P & P



Potable Water Demand Calculations

	CROZIER
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Project No.: 1733-6596 Project Name: Royal Rose Court Revision Date: 8-Mar-23 Prepared By: PM Checked By: NS/GC

-	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/dav)	Proposed Units (Person /	Proposed Flow (L/dav)	
4.	Dwellings						_		_
	b) 2 bedroom dwelling	1100	1	Dwelling		1100	0	0	1
	ıble 8.2.1.3.B				Sub-Total Residential	1100	L/d	0	Ļ
tem	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)	I
2.	Assembly Hall - per seat								
	a) No food service, or	8	1	Seat	0	0		0	
	 b) Food service provided 	36	1	Seat	0	0	242	8712	
12.	Food Service Operations								
	 Restaurant (not 24 hour), per seat 	125	1	Seat	0	0	0	0	
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	0	0	
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0	
13.	Hospitals - per bed								
	 a) Including laundry facilities, or 	750	1	Bed	0	0	0	0	
	 b) Excluding laundry facilities 	550	1	Bed	0	0	0	0	
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0	
15.	Office Building ⁽³⁾								
	b) Per each 9.3 m ² of floor space	75	9.3	m ²	0	0	0	0	1
26.	b) Per loading bay	150	1	Loading Bay		0		0	1
					Sub-Total Non-Residential	18000	L/d	8712	1
					Total	19100	L/d	8712	

	CONSULTING ENGINEERS					F 1.11	Project No. Project Name Revision Date Prepared By Checked By	: Royal Rose : 8-Mar-23 : PM : NS/GC
	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/dav)	Proposed Units (Person /	Proposed Flow (L/dav)
4.	Dwellings							
	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0
	able 8.2.1.3.B				Sub-Total Residential	1100	L/d	0
	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)
2.	Assembly Hall - per seat							
	a) No food service, or	8	1	Seat	0	0		0
	 b) Food service provided 	36	1	Seat	0	0	0	0
12.	Food Service Operations							
	a) Restaurant (not 24 hour), per seat	125	1	Seat	0	0	172	21500
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	0	0
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0
13.	Hospitals - per bed							
	a) Including laundry facilities, or	750	1	Bed		0		0
	b) Excluding laundry facilities	550	1	Bed		0		0
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0
15.	Office Building ⁽³⁾							
		75		2	_			

b) Per each 9.3 m² of floor space

9.3

 m^2

75

0

Total

Sub-Total Non-Residential 18000

0

19100

180

L/d

L/d

1452

22952 L/d

22952 L/d

(C) CROZIER						Project No.: Project Name: Revision Date: Prepared By:	Royal Rose 8-Mar-23
	CONSULTING ENGINEERS						Checked By:	
	able 8.2.1.3.A							
	Column 1	Column 2	Per	Unit	Existing Units (Person /	Existing	Proposed	Proposed
	Residential Occupancy	Volume,			Dwelling / m ² / etc)	Flow	Units	Flow
4.	Dwellings	litres			2	(L/dav)	(Person /	(L/dav)
4.	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0
		1100			Sub-Total Residential	1100	L/d	0
BC To	able 8.2.1.3.B							
	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)
2.	Assembly Hall - per seat							
	a) No food service, or	8	1	Seat	0	0		0
	 b) Food service provided 	36	1	Seat	0	0	0	0
12.	Food Service Operations							
	 a) Restaurant (not 24 hour), per seat 	125	1	Seat	0	0	172	21500
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	127	15875
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0
13.	Hospitals - per bed							
	 a) Including laundry facilities, or 	750	1	Bed	0	0	0	0
	b) Excluding laundry facilities	550	1	Bed	0	0	0	0
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0
15.	Office Building ⁽³⁾							
	b) Per each 9.3 m ² of floor space	75	9.3	m ²	0	0	0	0
	•				Sub-Total Non-Residentia	18000	L/d	37375

Total 19100 L/d

37375 L/d

Project No.: 1733-6596 Project Name: Royal Rose Court Revision Date: 08-Mar-23 Prepared By: PM Checked By: NS/GC

ltem	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person /	Proposed Flow (L/day)	I
4.	Dwellings						-		
	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0	
					Sub-Total Residential	1100	L/d	0	L
OBC Table 8.2.1.3.B Item Column 1 Establishments		Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)	1
2.	Assembly Hall - per seat								
	a) No food service, or	8	1	Seat	0	0	0	0	
	b) Food service provided	36	1	Seat	0	0	358	12888	
12.	Food Service Operations								
	a) Restaurant (not 24 hour), per seat	125	1	Seat	0	0	172	21500	
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	127	15875	
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0	
13.	Hospitals - per bed								
	a) Including laundry facilities, or	750	1	Bed	0	0	0	0	
	b) Excluding laundry facilities	550	1	Bed	0	0	0	0	
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0	
15.	Office Building ⁽³⁾								
	b) Per each 9.3 m ² of floor space	75	9.3	m²	0	0	0	0	
	·		•		Sub-Total Non-Residentia	18000	L/d	50263	ן י
					Total	19100	L/d	50263	L

J:\1700\1733-Fusioncorp Dev Inc\6596 - Royal Rose Court\Design\Civil_Water\Water & SAN\2023.03.07_OBC Servicing and SDU Calcs

CROZIER &ASSOCIATES Consulting Engineers	Date:	1733-6596 2023.03.08 PM NS/GC
Royal Rose Court- Preliminary Water Design Flow (Existi	ng)	
Developed Site Area	0.65	ha
<u>Number of Residential Units</u> Residential Population (Governor's Residence) Commercial/Institutional Population (Jail)		persons persons
Total Population Total Design Water Flows	42	persons
Average Daily Residential Flow (Table 8.2.1.3.A Ontario Building Code)		L/sec
Average Daily Commercial/Institutional Flow (Table 8.2.1.3.B Ontario Building Code) Total Average Flow		L/sec L/sec
Max Day Peak Factor (Table 3-3 of MECP Design Guidelines for Drinking Water Systems) Max Day Demand Flow	9.03 2.00	L/sec
Peak Hour Factor (Table 3-3 of MECP Design Guidelines for Drinking Water Systems) Peak Hour Flow	13.59 3.00	L/sec

CROZIER &ASSOCIATES Consulting Engineers
--

File: 1733-6596 Date: 2023.03.08 By: PM Check By: NS/GC

Royal Rose Court- Preliminary Water Design Flow (Proposed)

Developed Site Area	0.65 ha
<u>Number of Residential Units</u> Commercial/Institutional Population Total Population	657 persons 657 persons
<u>Total Design Water Flows</u>	
Average Daily Commercial/Institutional Flow	0.58 L/sec
Total Average Flow	0.58 L/sec
Max Day Peak Factor	3.60
Max Day Demand Flow	2.09 L/sec
Peak Hour Factor	5.40
Peak Hour Flow	3.14 L/sec



Fire Flow Determination Per Fire Underwriters Survey (2020)

Water Supply for Public Fire Protection - 2020

Fire Underwitters Survey Part II - Guide for Determination of Fire Flows for Public Fire Protection in Canada

An estimate of fire flow required for a given are	
	RFF = 220 * C * sqrt A
where:	
	RFF = the required fire flow in litres per minute (L/min)
	C = the construction coefficient is related to the type of construction of the building
	= 1.5 for Type V Wood Frame Construction
	= 0.8 for Type IV-A Mass Timber Construction
	= 0.9 for Type IV-B Mass Timber Construction
	= 1.0 for Type IV-C Mass Timber Construction
	= 1.5 for Type IV-D Mass Timber Construction
	= 1.0 for Type III Ordinary Construction
	= 0.8 for Type II Non-combustible Construction
	= 0.6 for Type I Fire Resistive Construction
	A = the total effective floor area (effective building area) in square metres (excluding basements at least
	50 percent below grade) in the building considered

STEP A: Construction Coefficient (C)

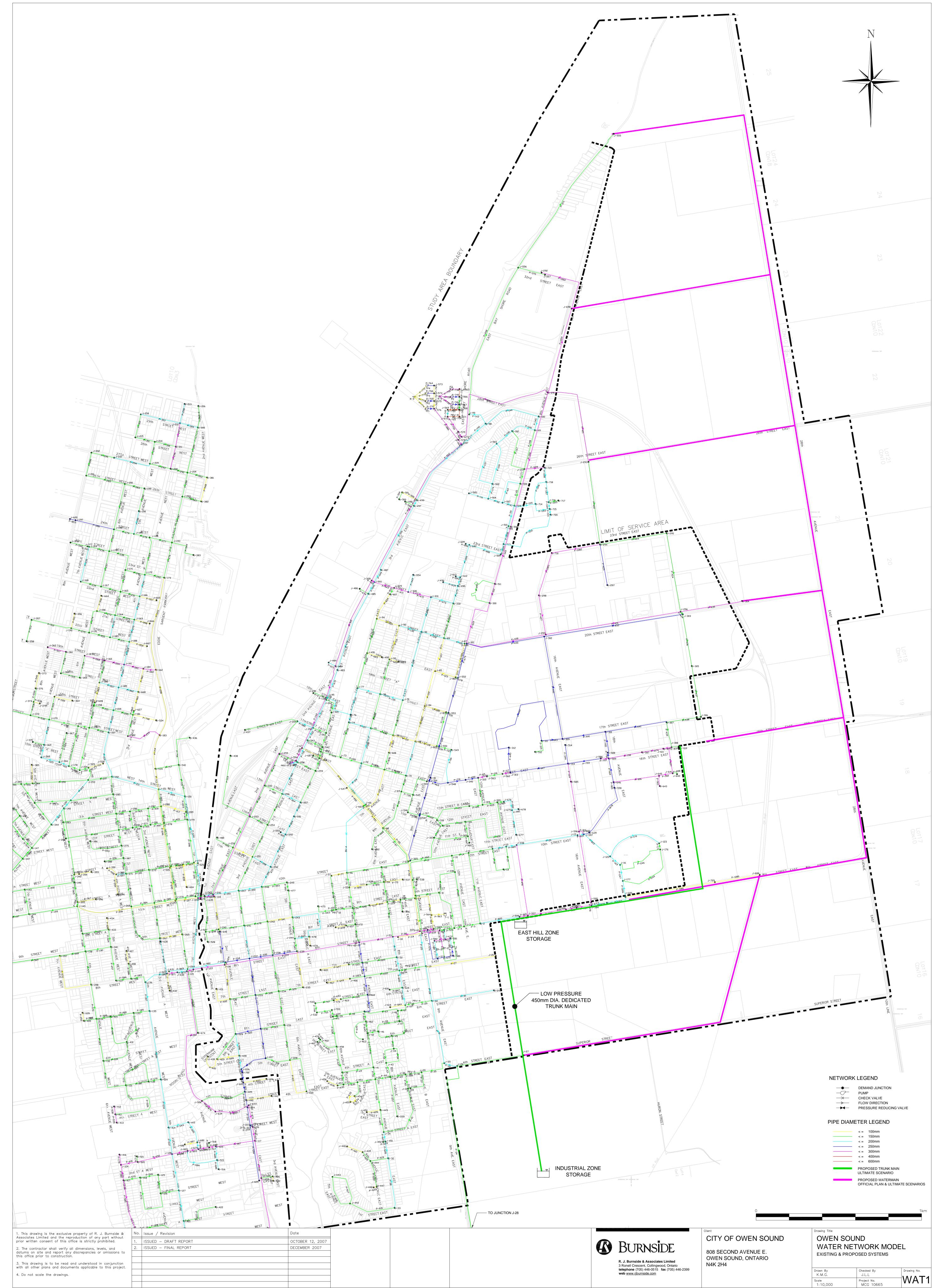
Type III Ordinary Construction

STEP B:	Total Effective Floor Area										
SIEP D.	Proposed Building	Abandoned courthouse to be rep		anua							
	Proposed Building	Floor areas recorded from "Conc									
			Yes/No/Unknown								
	Is baser	ment at least 50% below grade?	Yes	If yes, basement floor are	ea excluded						
Vertical openings protected? No *For consideration for effective area calculations											
	Calculate Effective Floor Area based on the highlighted cell										
		-C value below 1 and vertical op	penings are protected: Co	nsider single largest floor p	plus 25% of the two						
		immediately adjoining floors									
		*A building may be subdivided if	i there is a vertical firewall	with a fire-resistance ratin	a areater than 2 hours.						
		and meets the requirements of th			9 9 0 C C C C C C C C C C C C C C C C C						
		Floors Above Grade	Total Floor Area	% of Area Considered	Effective Floor Area						
			(m ²)		(m²)						
		Basement	201	0%	0.0	to Anality should blank					
		Gross Floor Area Total	2050 2050	100%	2050.0 2050.0	*Per Architectural Plans					
		Iolai	2050		2050.0						
		Total Effective Floor Area	2050) m²							
STEP C:		Therefore RFF =	10,000	L/min (rounded to neare	st 1000 L/min)						
TER D.	O										
TEP D:	Occupancy Contents Adjus	iment Factor									
	The required fire flow may b	be reduced by as much as -25% for a	occupancies having cont	ents with very low fire haz	ard or may be						
		charge for occupancies having a hig									
			Occupancy and Conten								
			Non-Combustible	-25%							
			Limited Combustible	-15% 0%							
			Combustible Free Burning	15%							
			Rapid Burning	25%							
*Refer to Table 3 for recommended Occupancy and Contents Charges by major occupancy examples.											
		Type of Occupancy	Adjustment								
		Assembly Occupancy	Limited Combustible	-15%							
		Total Reduction %	-1,500	L/min (reduction)							
		RFF =	8,500	L/min (not rounded)							



Fire Flow Determination Per Fire Underwriters Survey (2020)

STEP E:	Automatic Sprinkler Protection Sprinklers - The required fire flo		50% for complete automatic spr	inkler protection depe	nding upon adequacy c	of system.		
				Yes/No/Unknown	*Possible Reduction	Actual Reduction		
			in accordance with NFPA 13? d Fire Department hose lines? Fully supervised system?	Unknown Unknown Unknown	Available -30% -10% -10%	Provided 0% 0% 0%		
	*Reduction available assume *30% reduction typical for buil							
		Total Reduction %	0%	(reduction)				
		Total Reduced Flow	0	L/min (reduction, not ro	unded)			
STEP F:	Exposure Adjustment Charge							
	fire from the subject building t	to exposed risks. The required f	ce added to the required fire flo ire flow of a subject building ma arge considers the usage of wa	ay be increased deper	nding on the severity of e	exposed risks to the subject	t building and the dista	ince
			0 to 3m 3.1 to 10m 10.1 to 20m 20.1 to 30m	Maximum Exposure Adjustment Charge 25% 20% 15% 10% 20%				
	*The maximum exposure adju *The distance in metres from t	istment charge to be applied he subject building facing wa e subject building or the expos	g of no less than 2 hours, then th to a subject building is 75% II to the exposed building facing ed building is at a diagonal to	g wall, measured to the	e nearest metre, betwee	en the nearest points of		
	Exposed buildings		Distance	Surcharge Factor	Surcharge (L/min)			
		Adjacent Dwelling Adjacent Dwelling	26 37	10% 0%	850 C			
	South	Adjacent Institutional Adjacent Dwelling	2 42	25% 0%	2125	5		
		Total Reduced Flow	2,975	L/min Surcharge (not ro	ounded)			
STEP G:	Final Required Fire Flow							
	Step D - Occupanc	y Adjusted Fire Flow Demand	8,500 I			Table 1 - FUS 2020		_
		Step E - Sprinkler (Reduction)	0 L/min 2,975 L/min			Required Dura	ion of Fire Flow	
		Step F - Exposure Charge	2,773	_/ min		Flow Required (L/min)	Duration (hours)	
		Final Fire Flow:	11,475	L/min		2,000 or less	1.00	
				L/min (rounded to near	est 1000L/min)	3,000	1.25	
		or	183 1			4,000	1.50	
		or Required duration:	2,906			5,000 6,000	1.75 2.00	
		required duidtion:	*Refer to Table 1 for Duration	10013		8,000	2.00	
						10,000	2.00	
1						12,000	2.50	
1						14,000	3.00	
1						16,000	3.50	
1						18,000	4.00	
1						20,000 22,000	4.50 5.00	
1						24,000	5.50	
1						24,000	6.00	
						28,000	6.50	
1						30,000	7.00	
1						32,000	7.50	
1						34,000	8.00	
1						36,000	8.50	
1						38,000	9.00 9.50	
						40,000 and over *Interpolate for intermed		L
						interpolate for intermed	nare ingules	
L								



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Fire Flow Node FlexTable: Fire Flow Report Report

Label	Zone	Fire Flow (Needed) (L/s)	Fire Flow (Available) (L/s)	Pressure (Calculated Residual Lower Limit)	Junction w/Minimum Pressure (Zone)	Pressure (Residual Lower Limit) (psi)
				(psi)		
J-251	Spring	227.0000	153.2162	31.1	J-478	20.0
J-252	Spring	227.0000	151.8341	31.5	J-478	20.0
J-253	Spring	227.0000	151.0734	20.0	J-438	20.0
J-254	Spring	227.0000	144.2366	20.0	J-478	20.0
J-255	Spring	227.0000	196.1942	20.0	J-478	20.0
J-256	Spring	227.0000	228.3143	27.2	J-478	20.0
J-257	Spring	227.0000	199.2494	20.0	J-591	20.0
J-258	Spring	227.0000	385.8813	33.1	J-688	20.0
J-259	Spring	227.0000	159.2749	20.0	J-688	20.0
J-260	Spring	227.0000	197.7299	25.8	J-261	20.0
J-261	Spring	227.0000	147.4473	20.0	J-601	20.0
J-262	Spring	227.0000	154.0893	22.1	J-601	20.0
J-263	Spring	227.0000	92.3967	28.7	J-282	20.0
J-264	Spring	227.0000	100.9409	30.5	J-432	20.0
J-265	Spring	227.0000	86.2573	21.2	J-282	20.0
J-267	Spring	227.0000	74.7711	20.0	J-282	20.0
J-268	Spring	227.0000	66.7131	20.0	J-282	20.0
J-269	Spring	227.0000	75.5471	23.7	J-282	20.0
J-270	Spring	227.0000	80.9376	26.9	J-282	20.0
J-271	Spring	227.0000	64.2104	26.0	J-390	20.0
J-272	Spring	227.0000	69.9967	27.1	J-282	20.0
J-273	Spring	227.0000	72.3029	27.5	J-282	20.0
J-274	Spring	227.0000	73.9415	26.1	J-282	20.0
J-275	Spring	227.0000	56.8105	27.0	J-386	20.0
J-276	Spring	227.0000	57.3883	28.7	J-282	20.0
J-277	Spring	227.0000	62.2800	28.4	J-282	20.0

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Bentley Systems, Inc. Haestad Methods Solution Center Bentley WaterCAD V8 XM Edition [08.09.165.00]

10/12/2007

27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Page 10 of 26



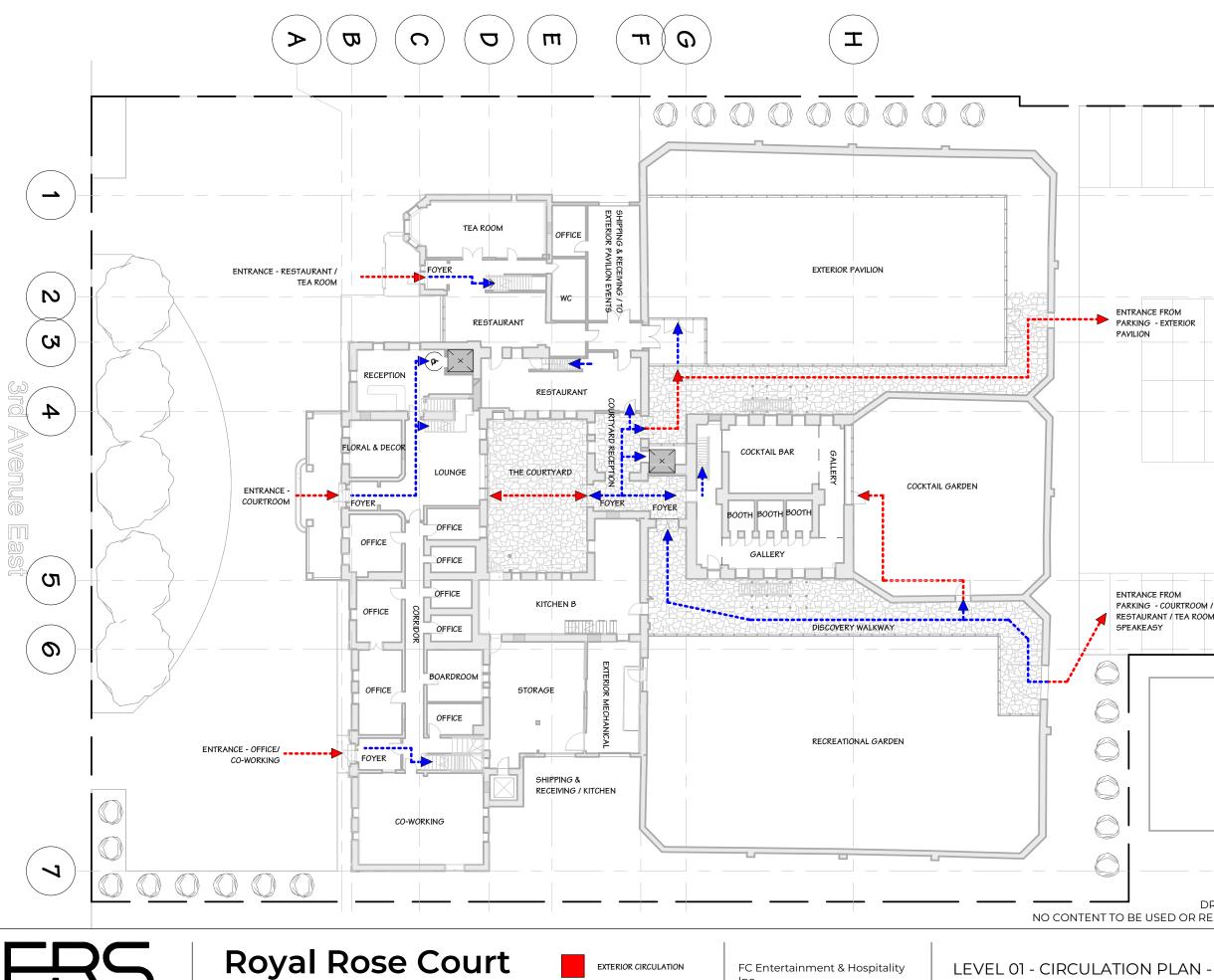
Royal Rose Court - Occupancy Plans (ERS Architects, December 2022)



Project # 22.0056.00

Royal Rose Court

Occupancy & Circulation Discussion December 22, 2022



1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

ARCHITECTS

INTERIOR CIRCULATION

Inc.

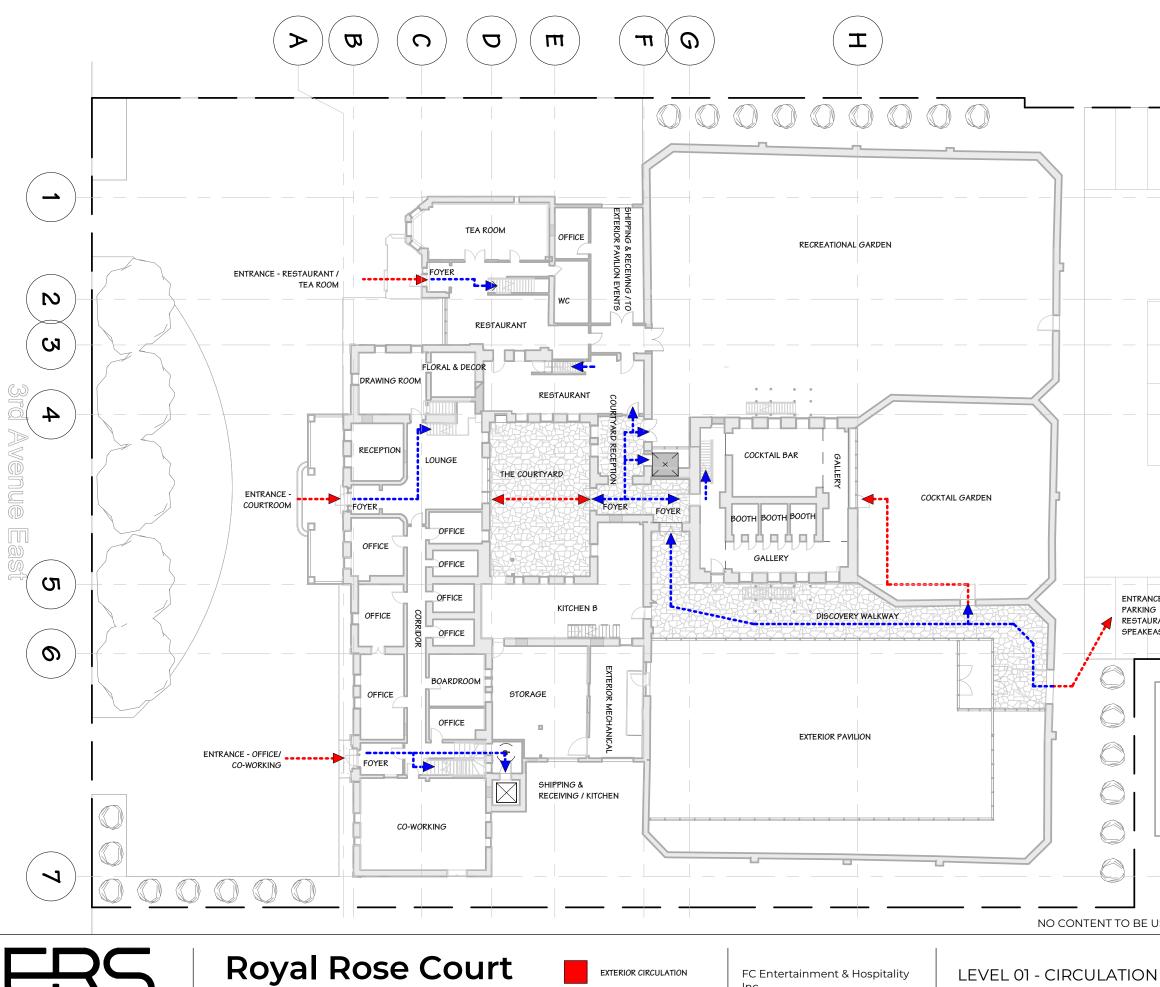
OPT 1

E FRO - EXT	M Erior											
											Ath /	
											4th Avenue East	
	M JRTROON TEA ROO										L N N N	
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יח		Proje	ct #	22	2.0056	.00		_				



As indicated 12/12/22

Scale Date



1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

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INTERIOR CIRCULATION

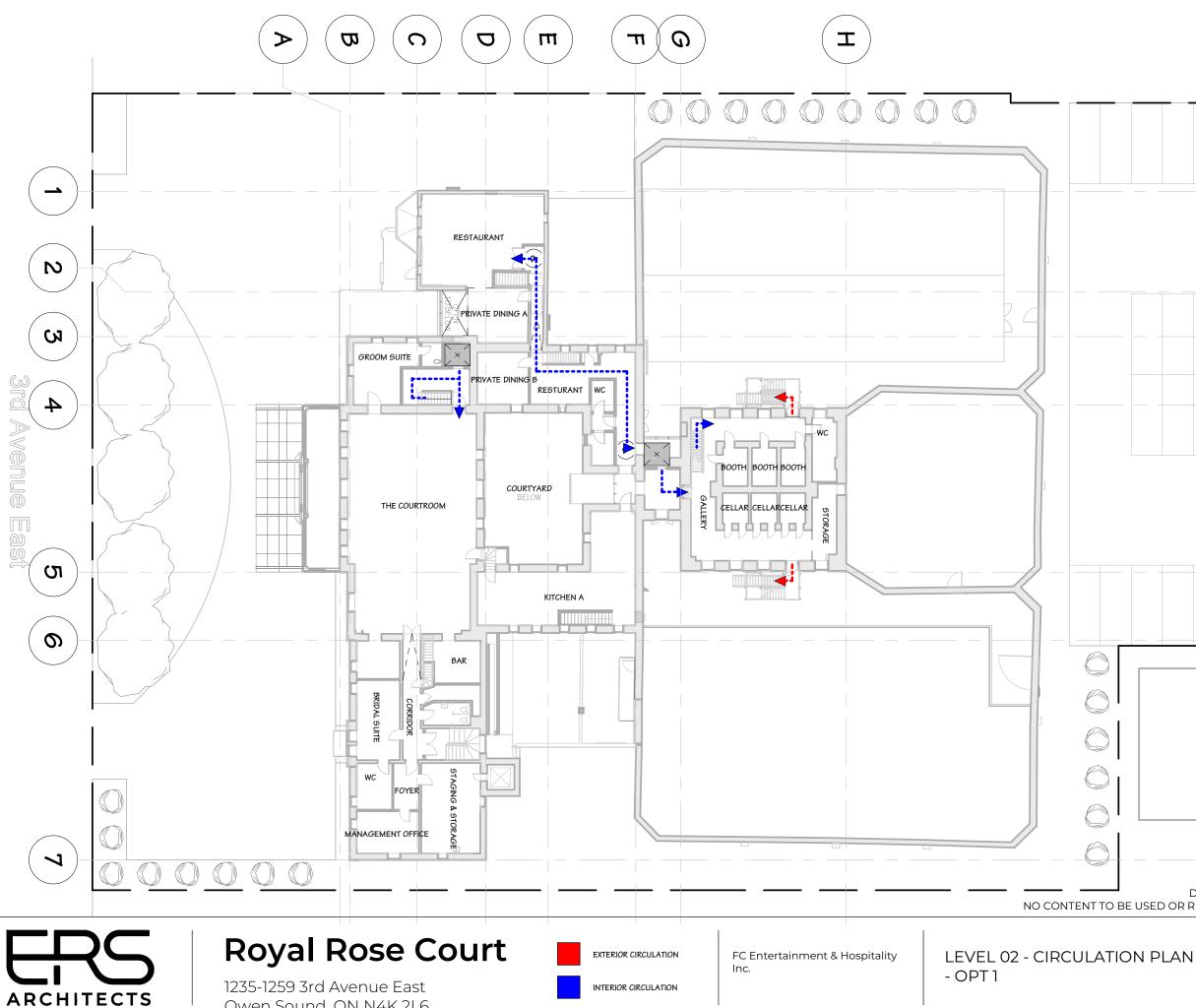
Inc.

LEVEL 01 - CIRCULATION OPT 2

			4th Avenue Ea:
E FROM - COURTROOM / ANT / TEA ROOM & SY			
			EPTUAL PURPOSES ONLY. ATION OF THE ARCHITECT.
PLAN -	-	2.0056.00 indicated	5K-001b

12/12/22

Date



Owen Sound, ON N4K 2L6

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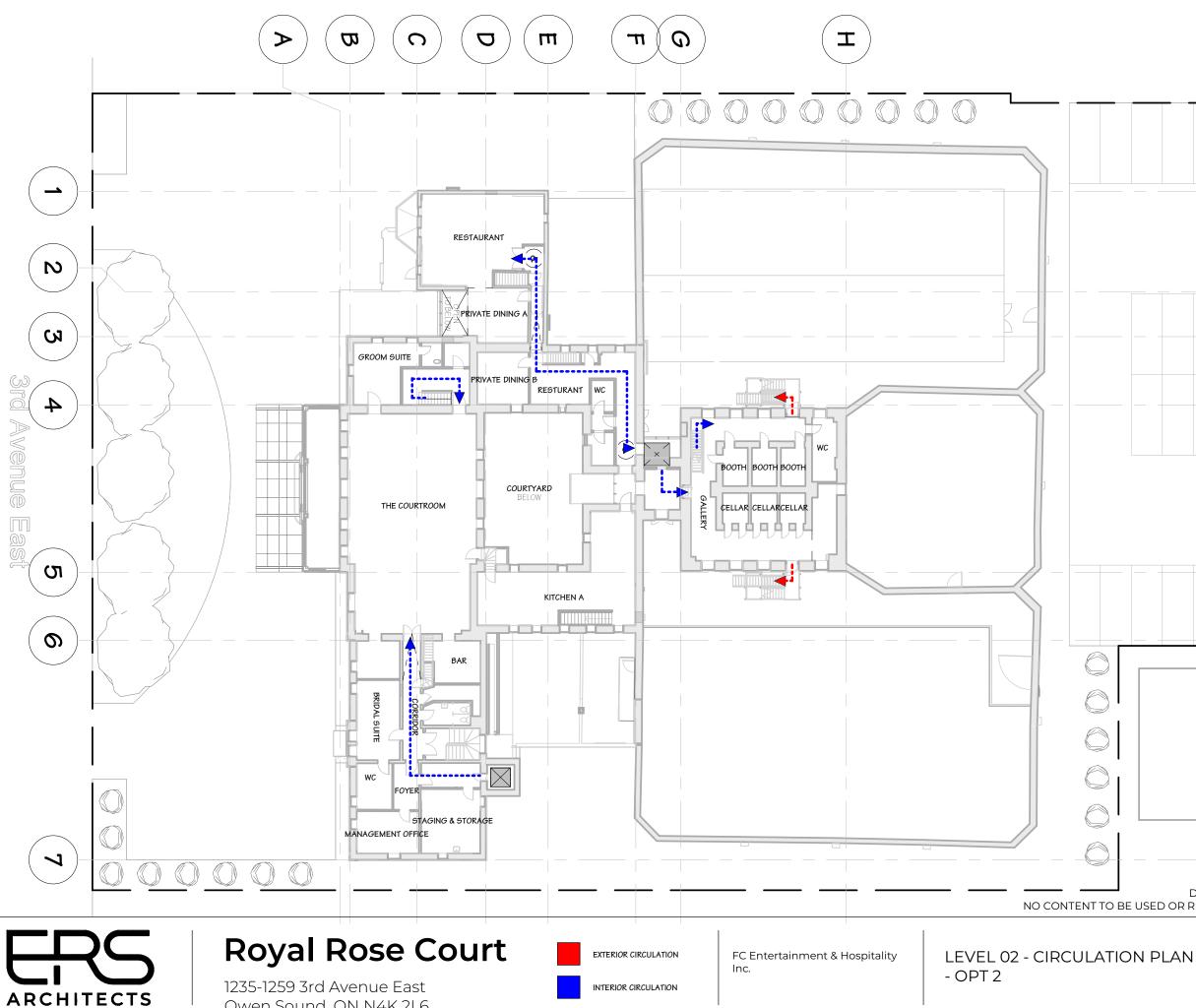
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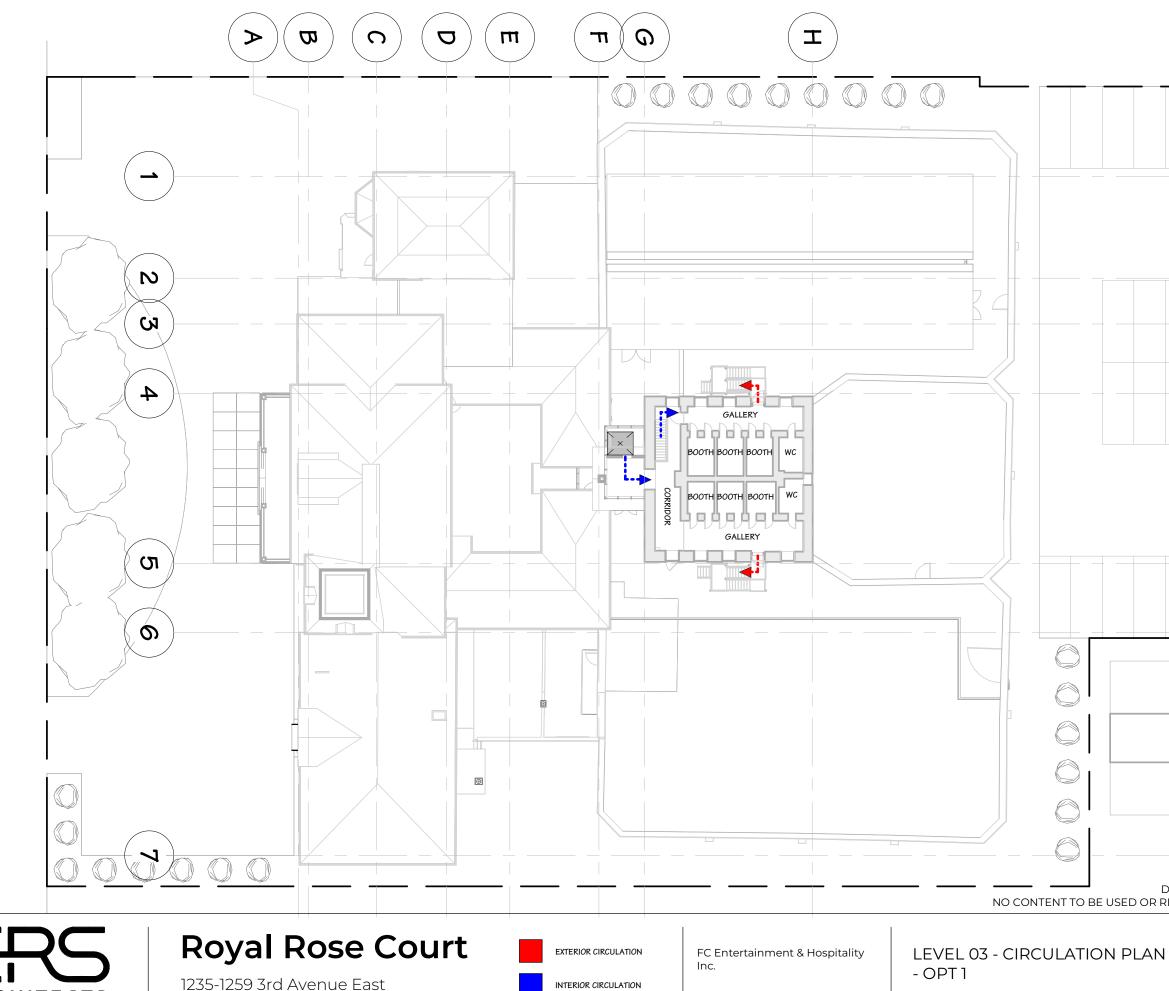
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INTERIOR CIRCULATION

3rd Avenue East

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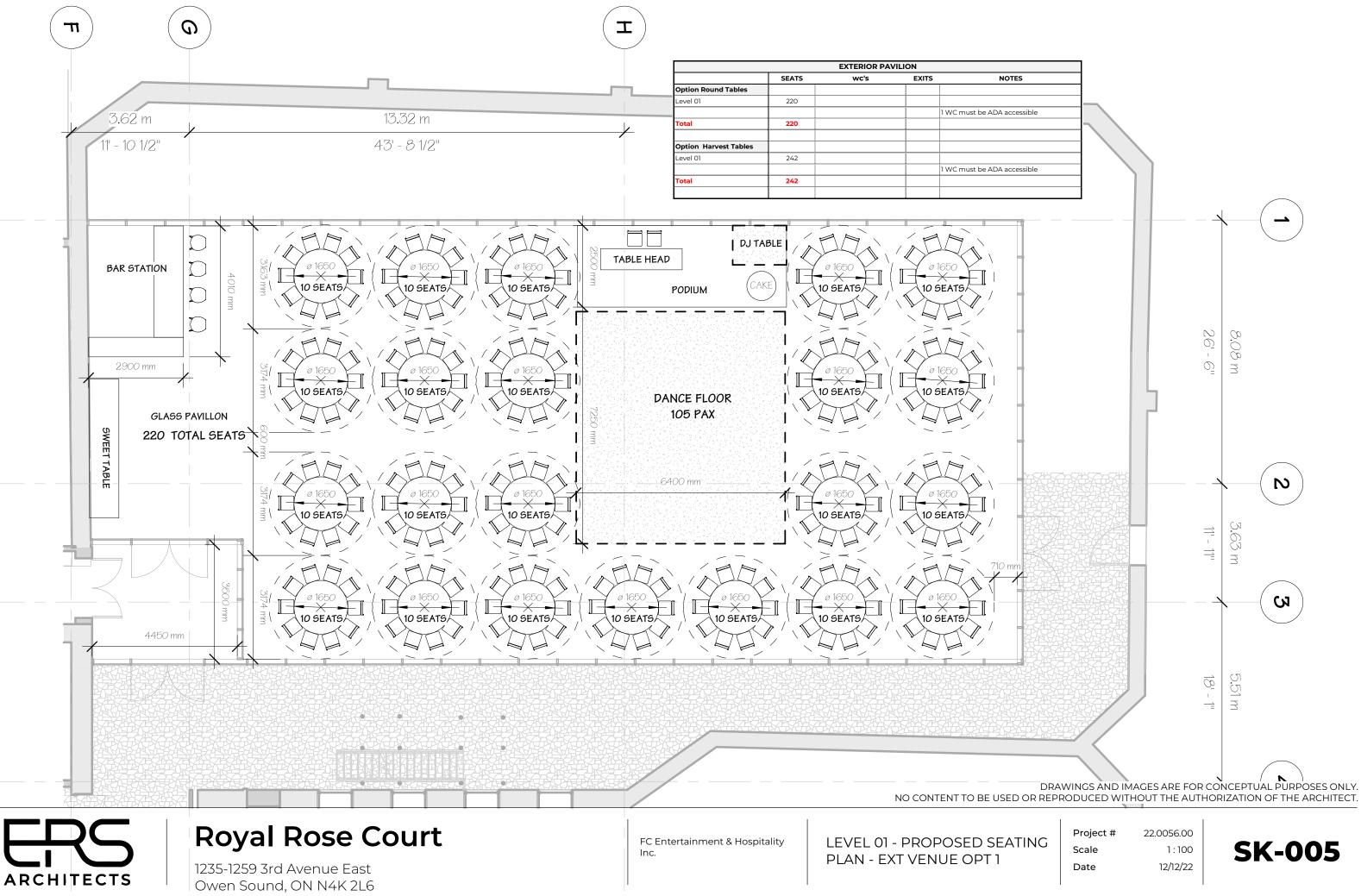
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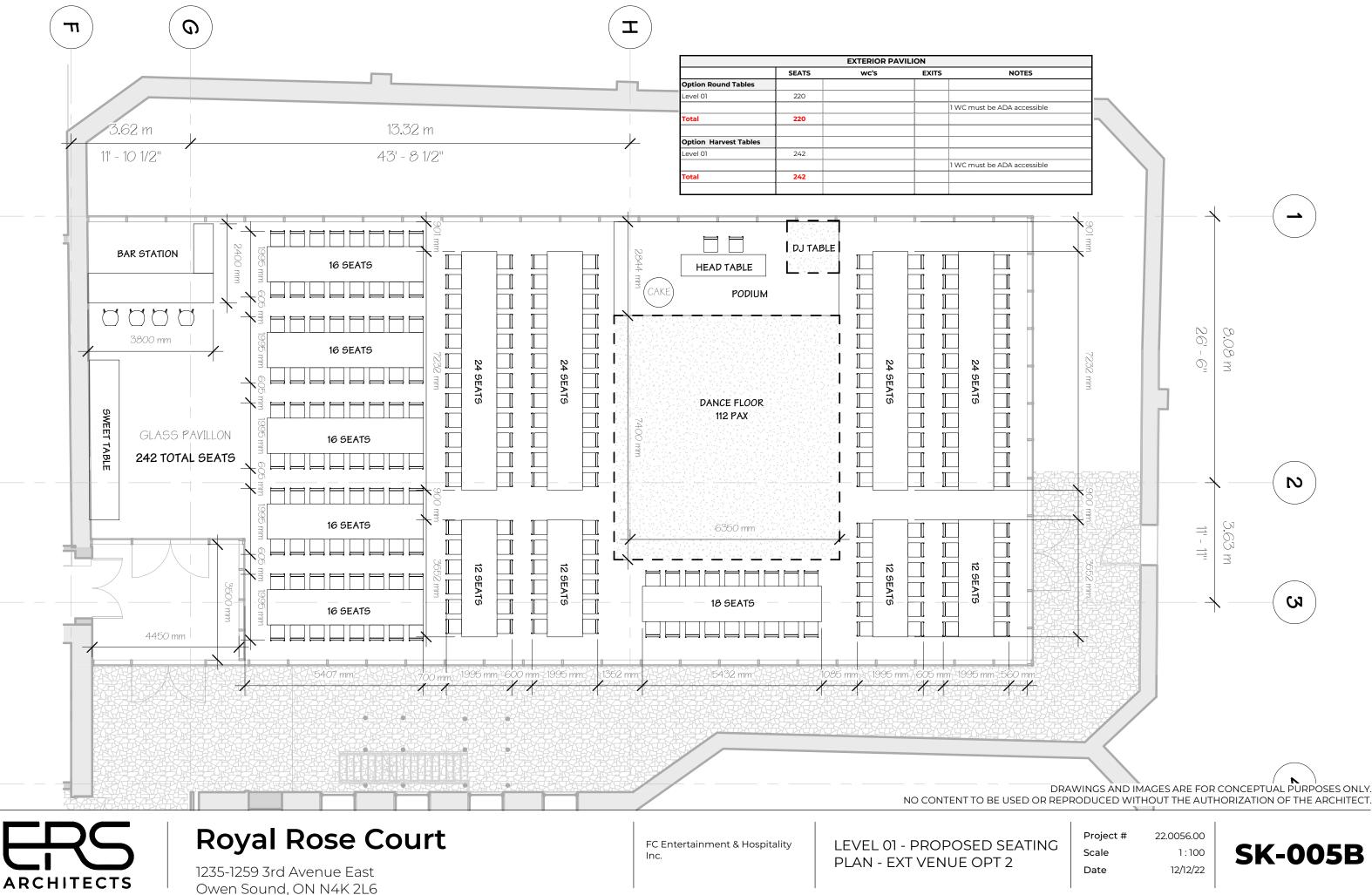


4th Avenue East

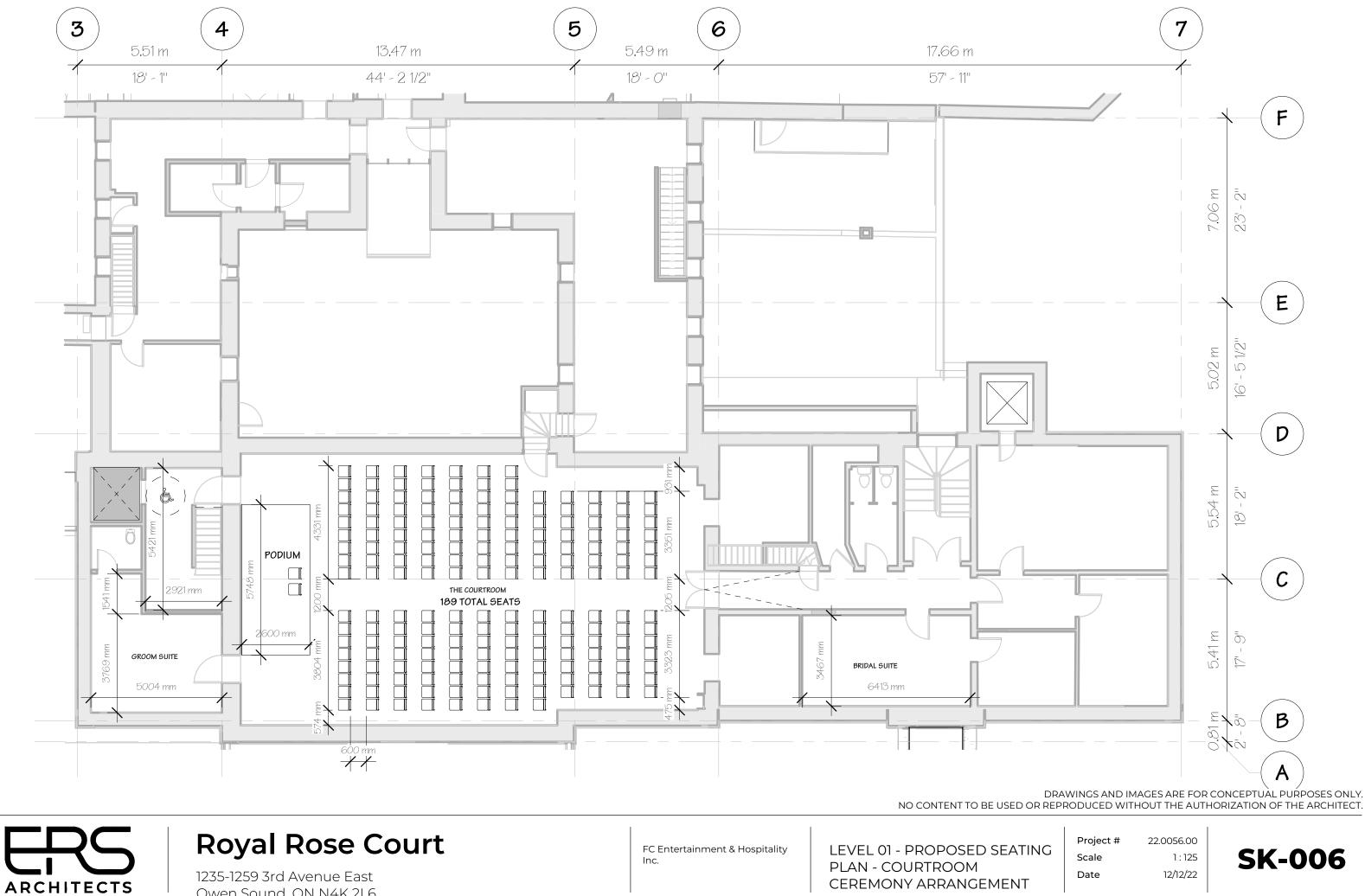
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Project # Scale Date





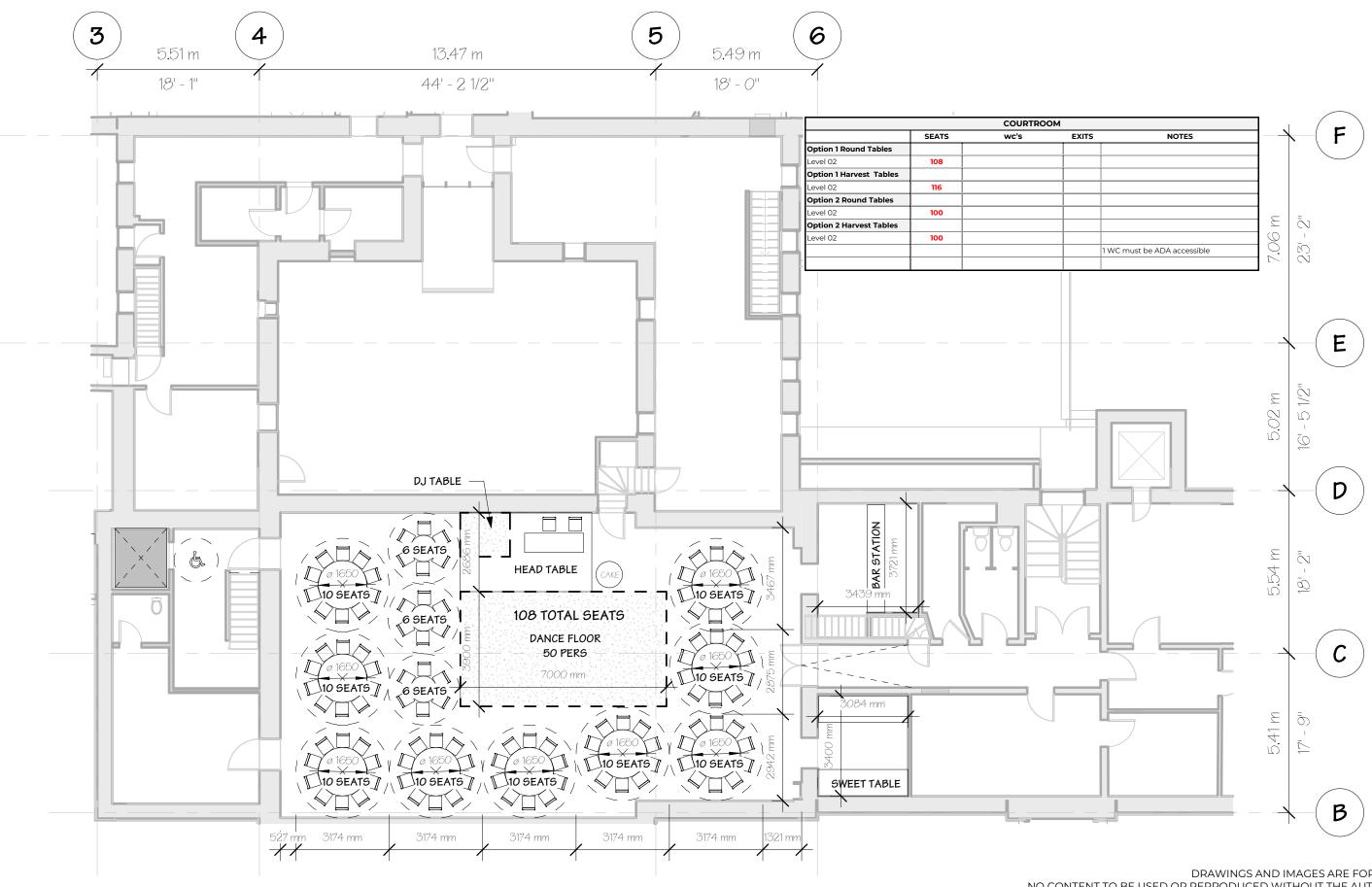
Owen Sound, ON N4K 2L6





Owen Sound, ON N4K 2L6

CEREMONY ARRANGEMENT



FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - COURTROOM OPT 1 Copy 1

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

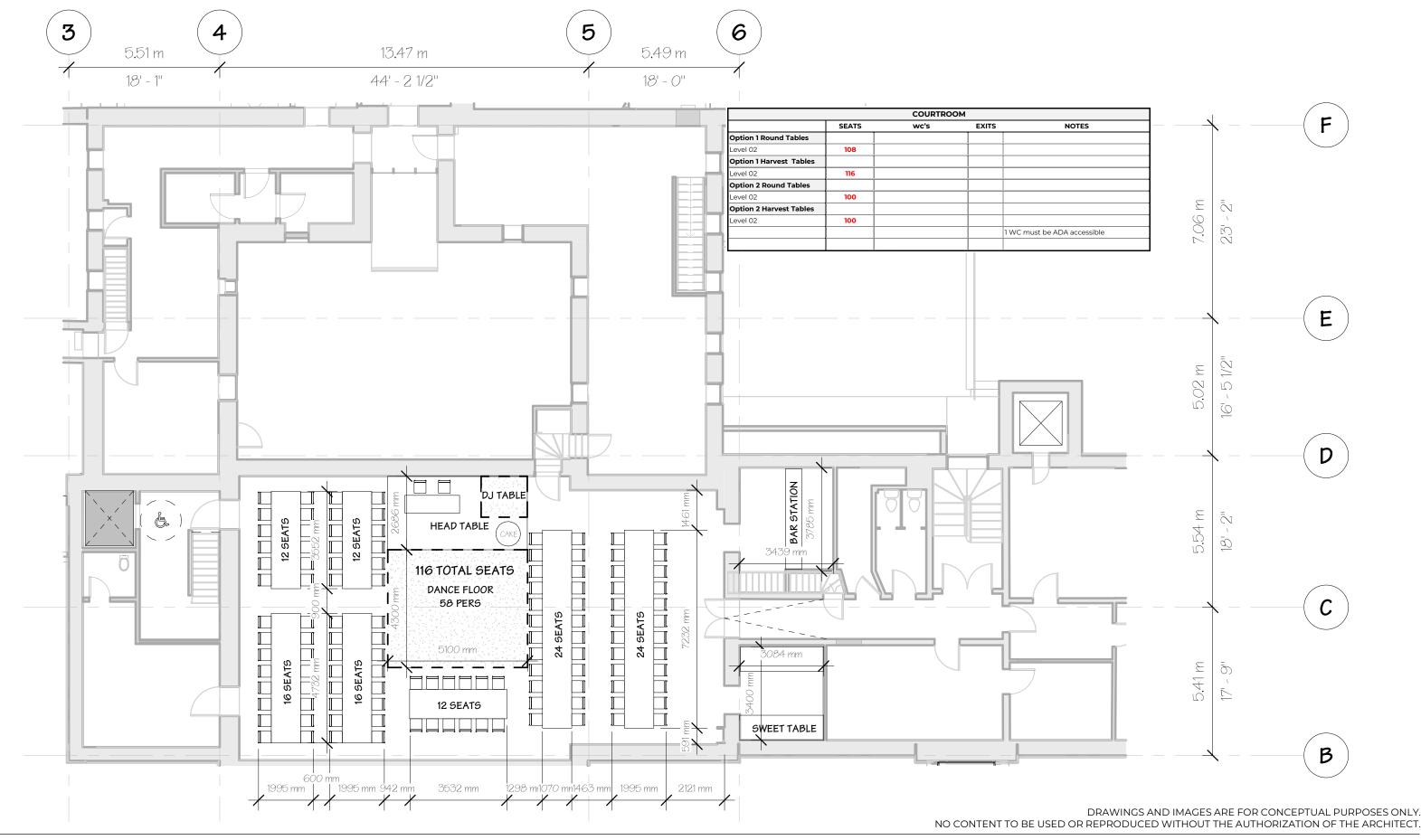
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Project # Scale Date

22.0056.00 1:125 12/12/22



FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - COURTROOM OPT 2

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

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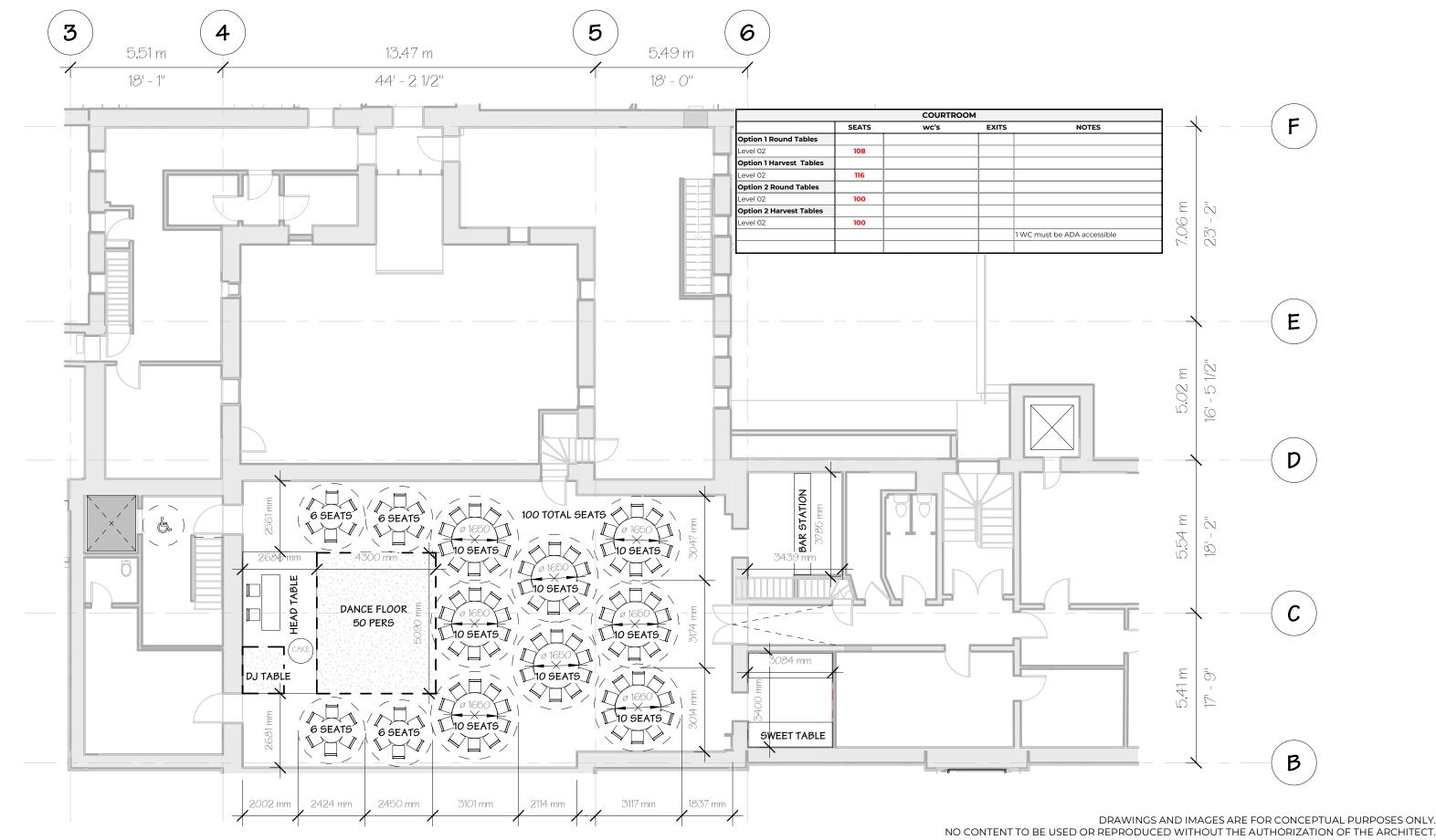
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Project # Scale Date

22.0056.00 12/12/22



FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - COURTROOM OPT 1

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

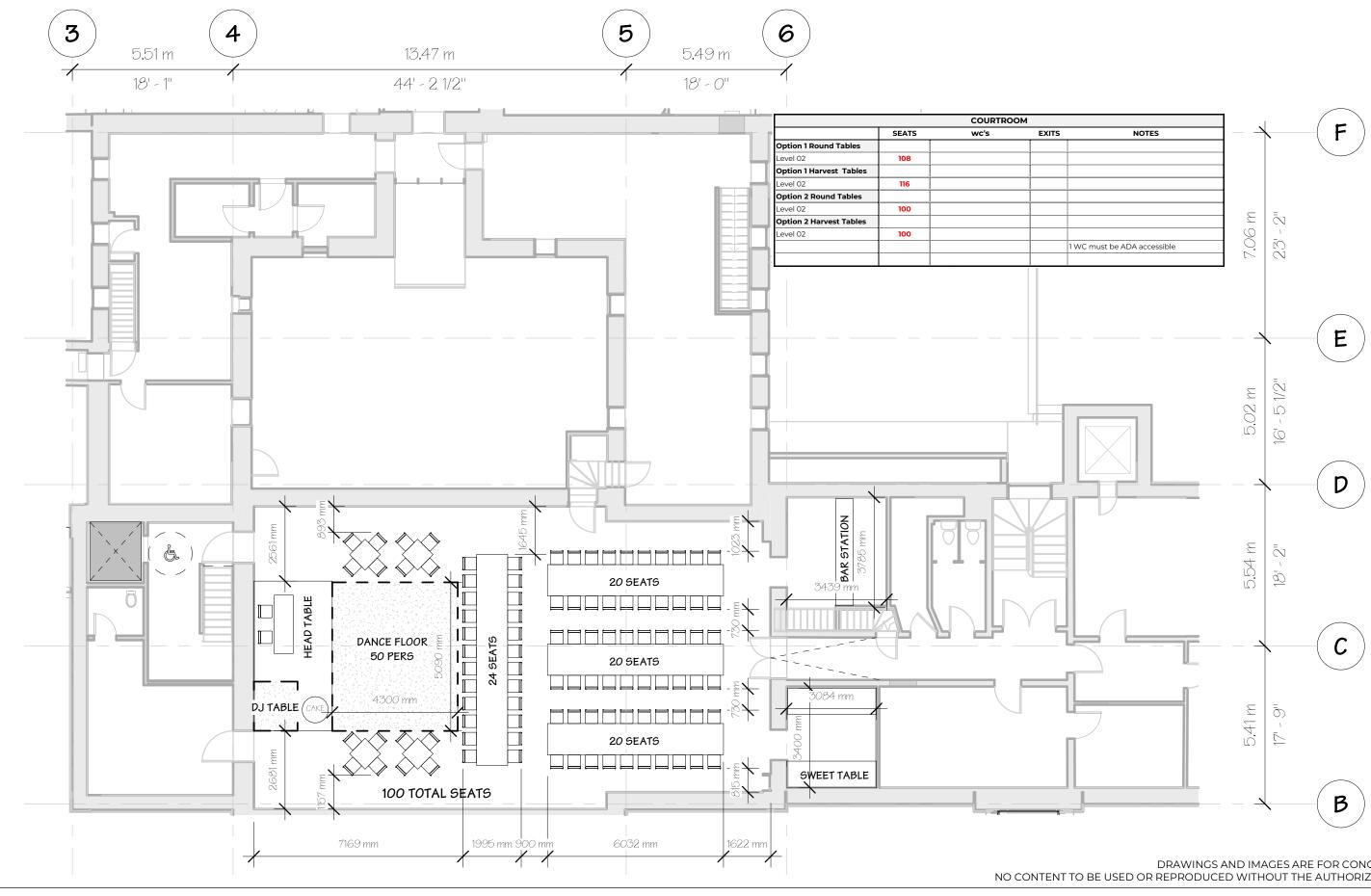
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22.0056.00 1:125 12/12/22



FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - COURTROOM OPT 1

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

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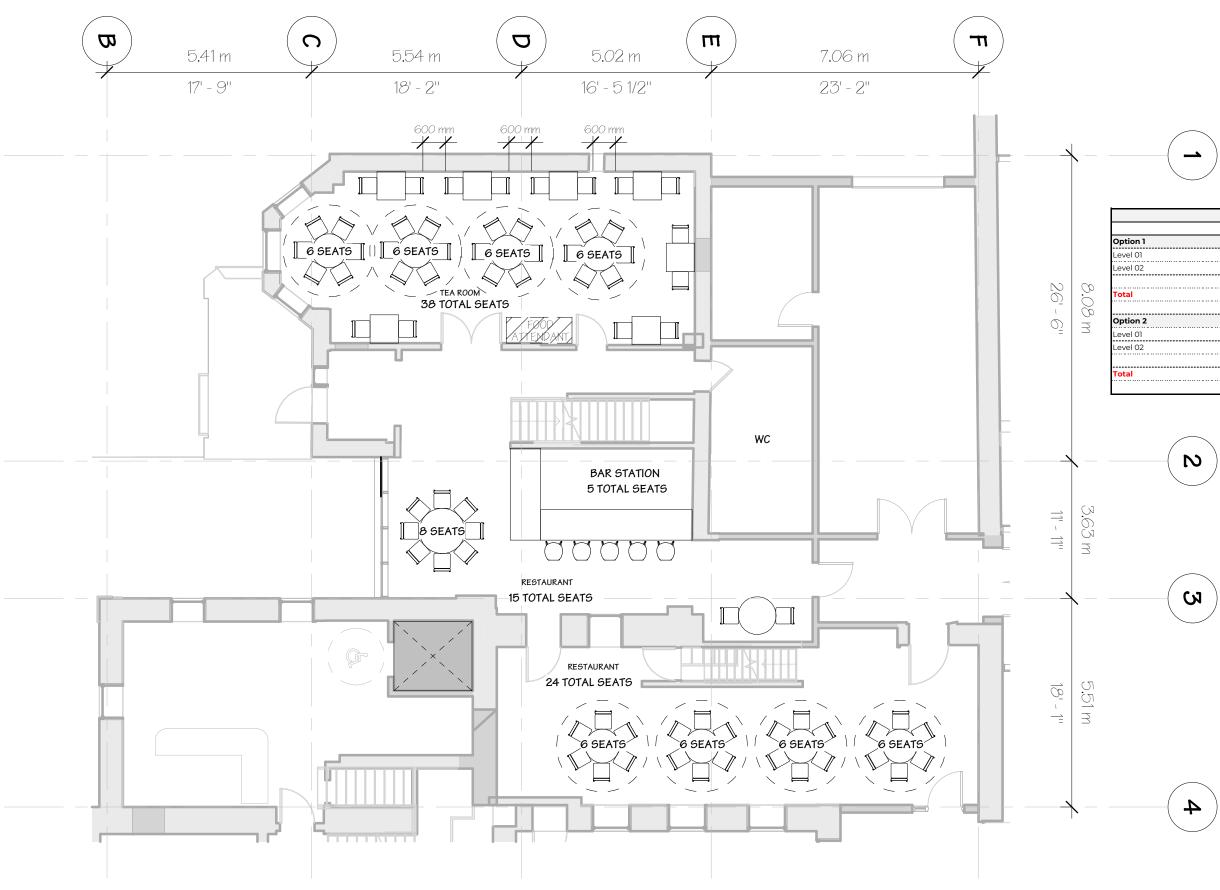
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12/12/22



22.0056.00 Project # 1:125 Scale

Date



1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

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FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - RESTAURANT **OPTION 1**

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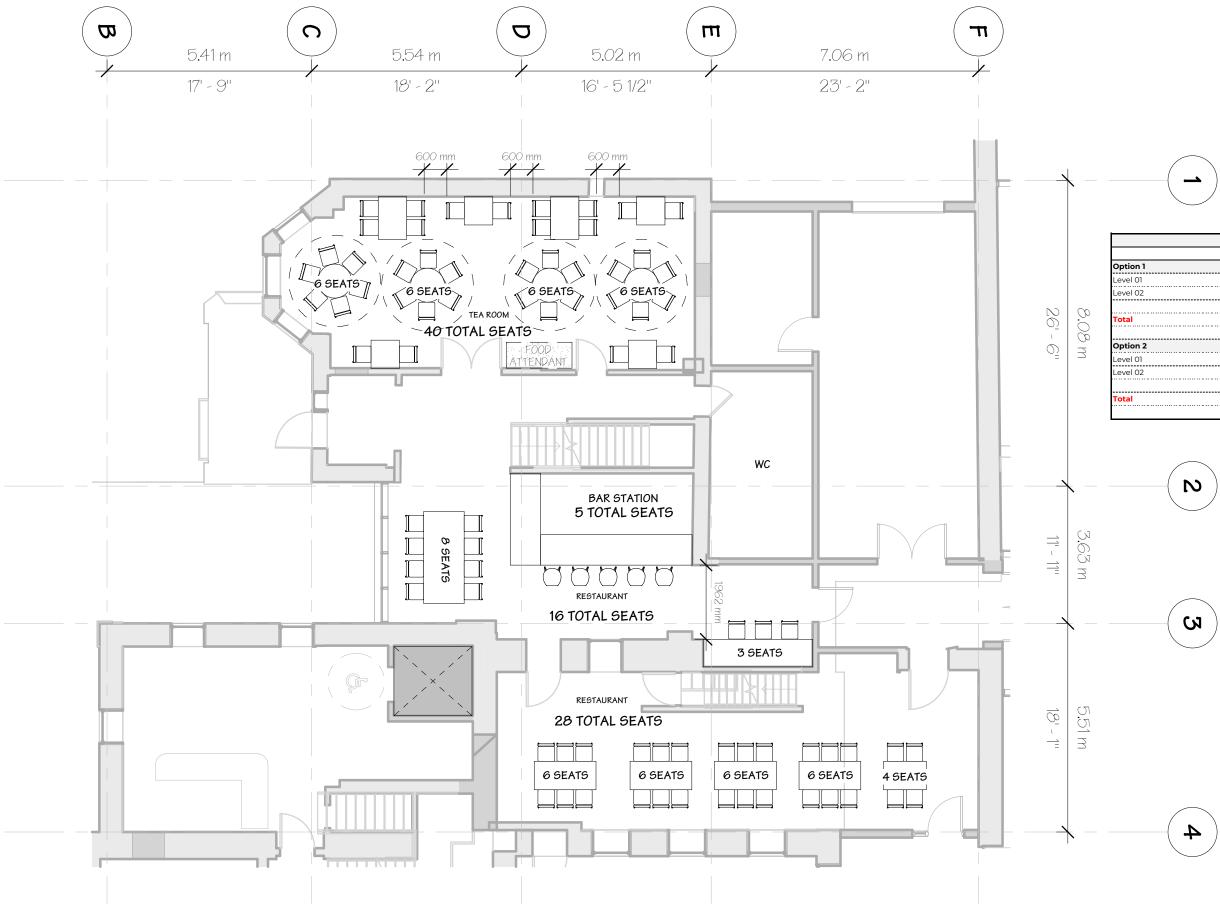
RESTAURANT / TEA HOUSE				
SEATS	wc's	EXITS	NOTES	
77	1	3		
 84	2			
			1 WC must be ADA accessible	
161	3			
 84	۱	3		
88	2			
			1 WC must be ADA accessible	
172	3			

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Royal Rose Court

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

ARCHITECTS

FC Entertainment & Hospitality Inc.

LEVEL 01 - PROPOSED SEATING PLAN - RESTAURANT **OPTION 2**

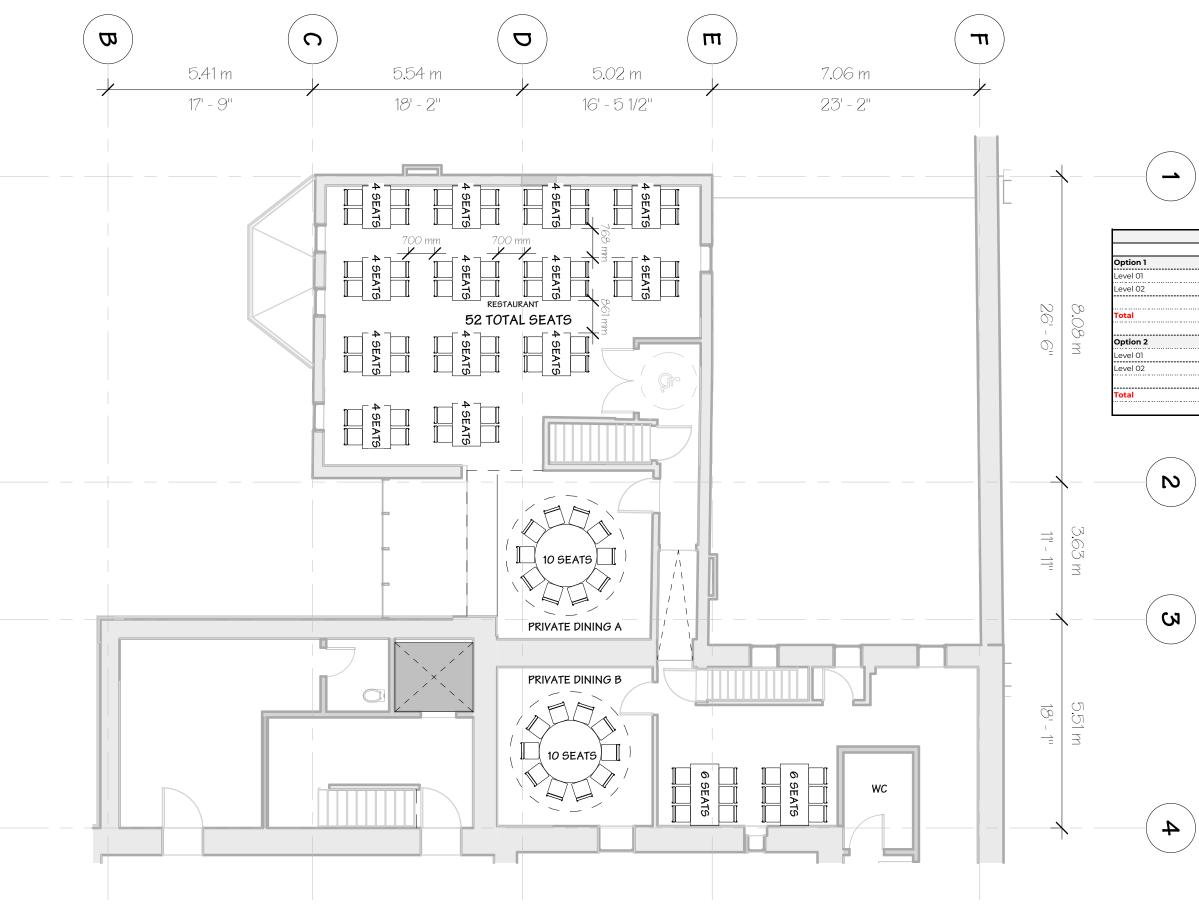
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RESTAURANT / TEA HOUSE				
SEATS	wc's	EXITS	NOTES	
77	1	3		
 84	2			
			1 WC must be ADA accessible	
161	3			
 84	۱	3		
88	2			
			1 WC must be ADA accessible	
172	3			



Project # Scale Date





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ARCHITECTS ROY 1235-12 Owen

Royal Rose Court

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

LEVEL 02 - PROPOSED SEATING PLAN - RESTAURANT OPTION 1

RESTAURANT / TEA HOUSE				
SEATS	wc's	EXITS	NOTES	
77	1	3		
 84	2			
			1 WC must be ADA accessible	
161	3			
84	1	3		
88	2			
			1 WC must be ADA accessible	
172	3			



Project # Scale Date



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ARCHITECTS

Royal Rose Court

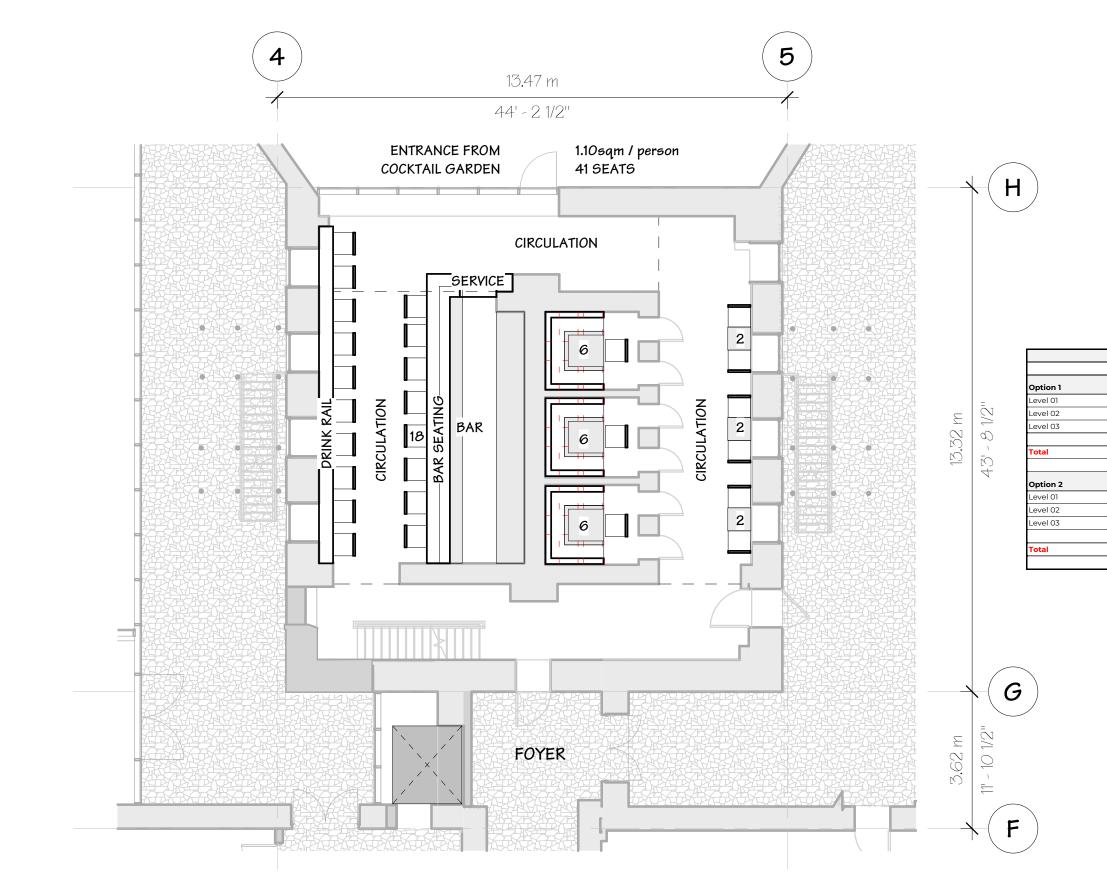
1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

LEVEL 02 - PROPOSED SEATING PLAN - RESTAURANT OPTION 2

RESTAURANT / TEA HOUSE					
SEATS	wc's	EXITS	NOTES		
77	1	3			
84	2				
			1 WC must be ADA accessible		
161	3				
84	1	3			
88	2				
			1 WC must be ADA accessible		
 172	3				



Project # Scale Date





1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

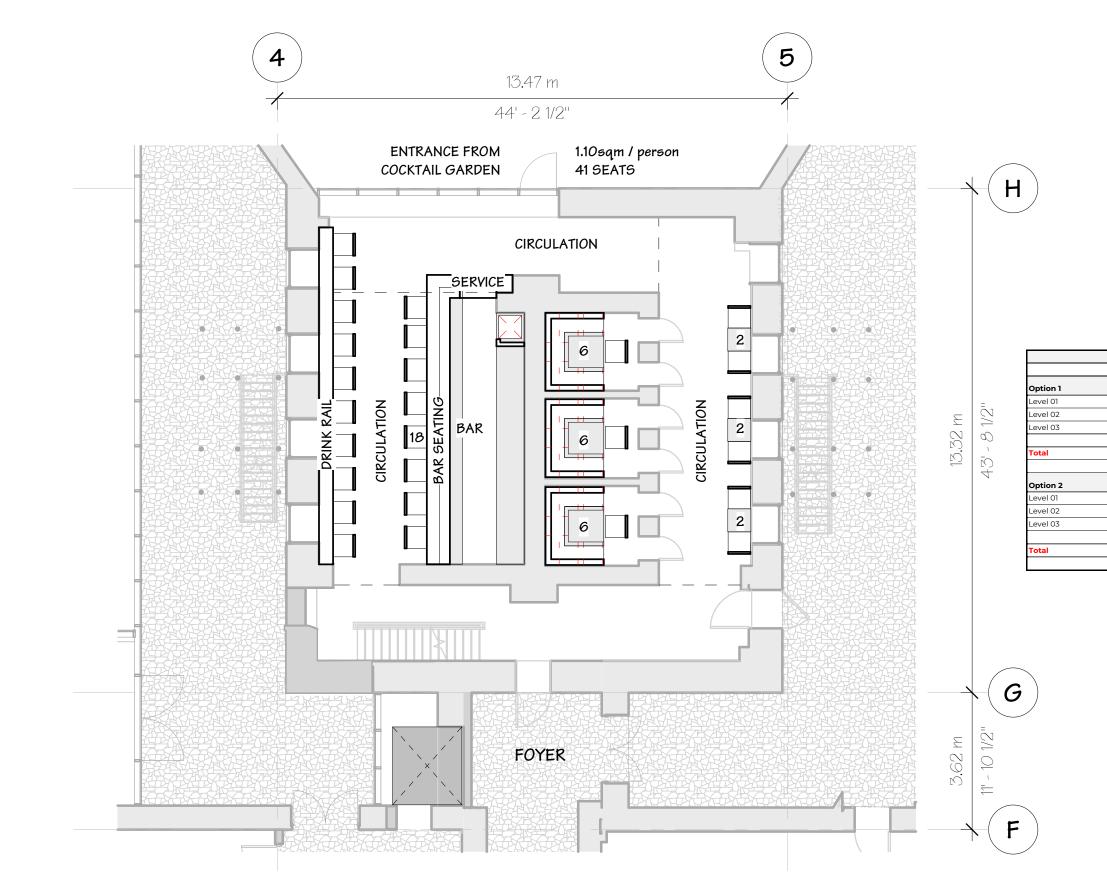
LEVEL 01 - PROPOSED SEATING PLAN - SPEAK EASY

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
 	unisex layout	0	
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
115	3		

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Project # Scale Date





1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

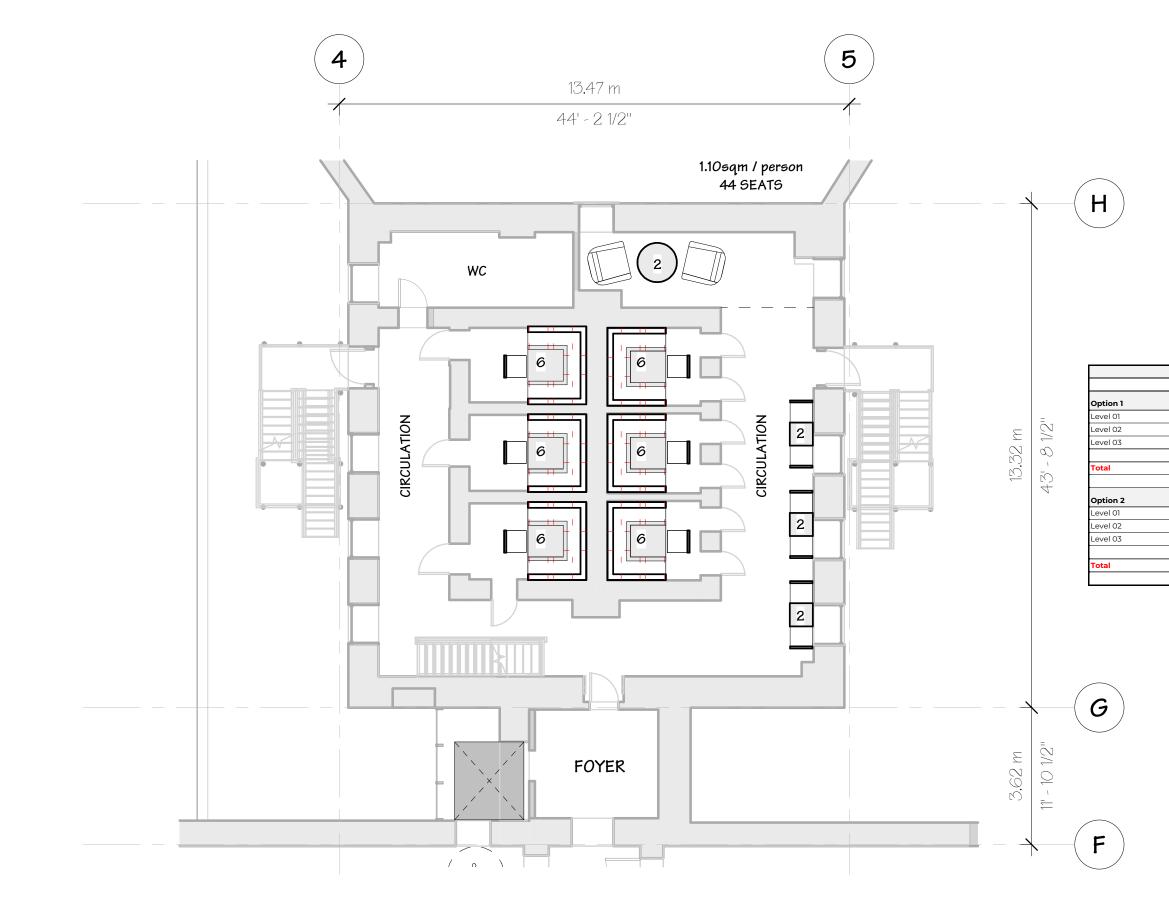
LEVEL 01 - PROPOSED SEATING PLAN - SPEAK EASY

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
 127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
 115	3		

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Royal Rose Court

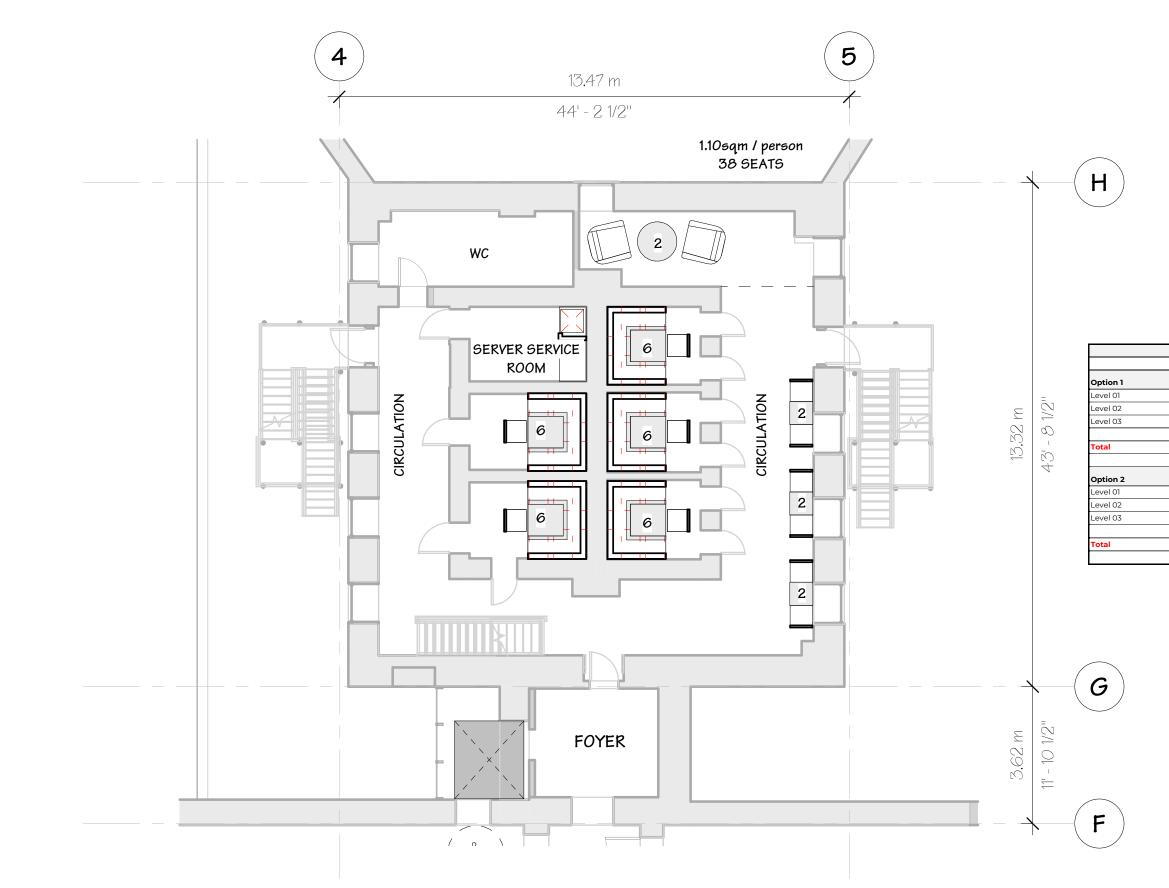
1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

LEVEL 02 - PROPOSED SEATING PLAN - SPEAK EASY

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
	unisex layout	0	
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
115	3		



Project # Scale Date



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Royal Rose Court

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

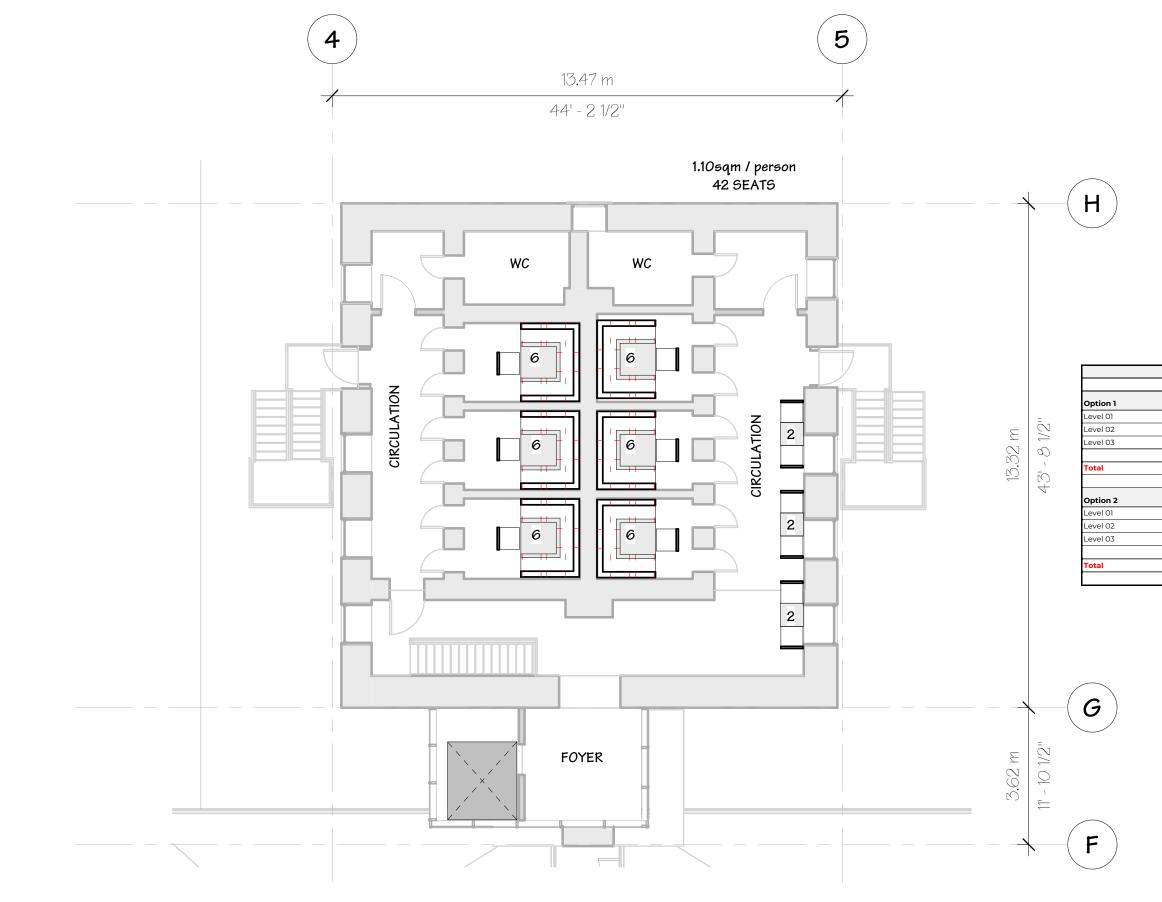
LEVEL 02 - PROPOSED SEATING PLAN - SPEAK EASY -Opt B

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
	unisex layout	0	
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
115	3		



Project # Scale Date





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Royal Rose Court

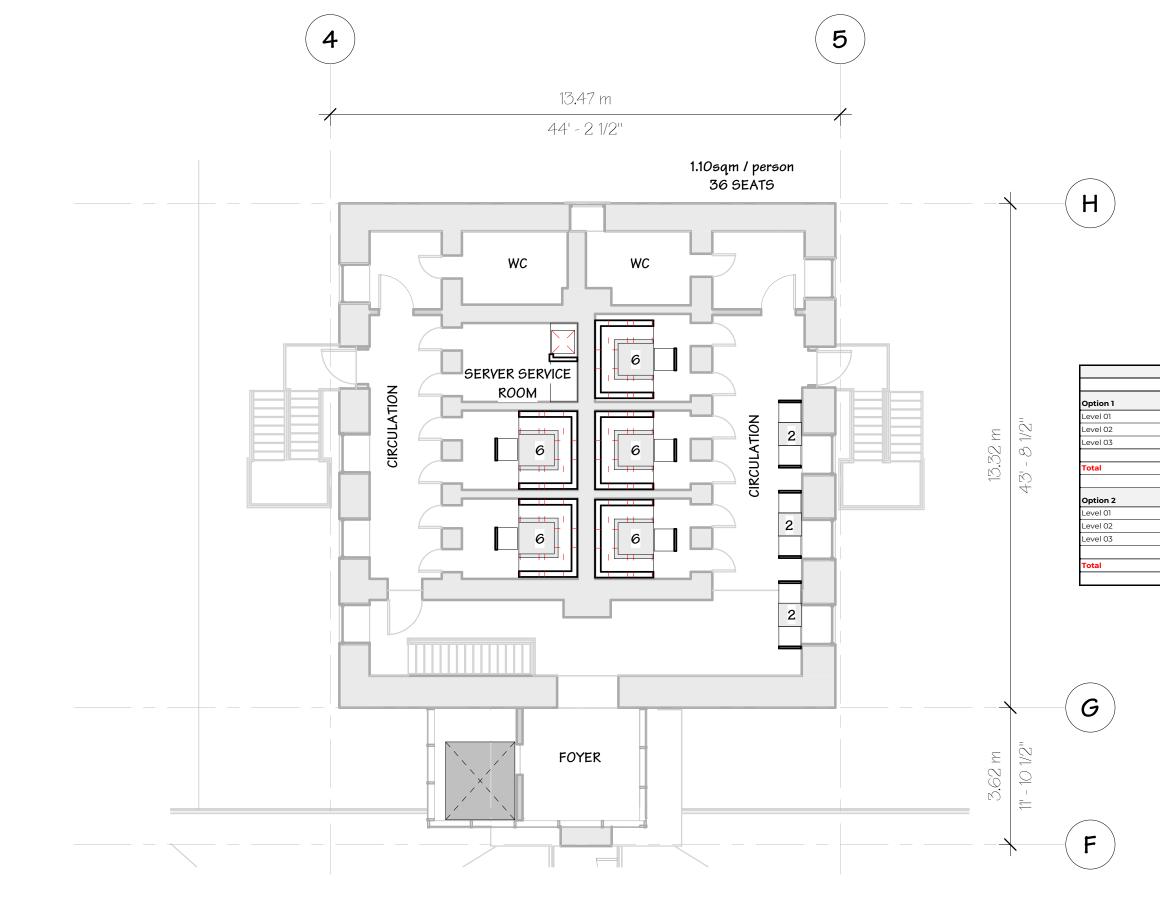
1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

LEVEL 03 - PROPOSED SEATING PLAN - SPEAK EASY

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
	unisex layout	0	
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
115	3		



Project # Scale Date



DRAWINGS AND IMAGES ARE FOR CONCEPTUAL PURPOSES ONLY. NO CONTENT TO BE USED OR REPRODUCED WITHOUT THE AUTHORIZATION OF THE ARCHITECT.



Royal Rose Court

1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6 FC Entertainment & Hospitality Inc.

LEVEL 03 - PROPOSED SEATING PLAN - SPEAK EASY -Opt B

	SPEAKEASY		
SEATS	wc's	EXITS	NOTES
	Require 6 WC's for ea. or		
	unisex layout	0	
41		2	
44	1	2	
42	2	2	
			1 WC must be ADA accessible
127	3		
	Require 6 WC's for ea. or		
	unisex layout		
41		2	
38	1	2	
36	2	2	
			1 WC must be ADA accessible
115	3		



Project # Scale Date



APPENDIX D

Sanitary Demand Calculations & PCSWMM Model

	CROZIER
--	---------

Project No.: 1733-6596 Project Name: Royal Rose Court Revision Date: 8-Mar-23 Prepared By: PM Checked By: NS/GC

-	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/dav)	Proposed Units (Person /	Proposed Flow (L/dav)	
4.	Dwellings						_		_
	b) 2 bedroom dwelling	1100	1	Dwelling		1100	0	0	1
	ıble 8.2.1.3.B				Sub-Total Residential	1100	L/d	0	Ļ
tem	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)	I
2.	Assembly Hall - per seat								
	a) No food service, or	8	1	Seat	0	0		0	
	 b) Food service provided 	36	1	Seat	0	0	242	8712	
12.	Food Service Operations								
	 Restaurant (not 24 hour), per seat 	125	1	Seat	0	0	0	0	
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	0	0	
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0	
13.	Hospitals - per bed								
	 a) Including laundry facilities, or 	750	1	Bed	0	0	0	0	
	b) Excluding laundry facilities	550	1	Bed	0	0	0	0	
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0	
15.	Office Building ⁽³⁾								
	b) Per each 9.3 m ² of floor space	75	9.3	m ²	0	0	0	0	1
26.	b) Per loading bay	150	1	Loading Bay		0		0	1
					Sub-Total Non-Residential	18000	L/d	8712	1
					Total	19100	L/d	8712	

	CONSULTING ENGINEERS					F 1.11	Project No. Project Name Revision Date Prepared By Checked By	: Royal Rose : 8-Mar-23 : PM : NS/GC
	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/dav)	Proposed Units (Person /	Proposed Flow (L/dav)
4.	Dwellings							
	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0
	able 8.2.1.3.B				Sub-Total Residential	1100	L/d	0
	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)
2.	Assembly Hall - per seat							
	a) No food service, or	8	1	Seat	0	0		0
	 b) Food service provided 	36	1	Seat	0	0	0	0
12.	Food Service Operations							
	a) Restaurant (not 24 hour), per seat	125	1	Seat	0	0	172	21500
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	0	0
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0
13.	Hospitals - per bed							
	a) Including laundry facilities, or	750	1	Bed		0		0
	b) Excluding laundry facilities	550	1	Bed		0		0
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0
15.	Office Building ⁽³⁾							
		75		2	_			

b) Per each 9.3 m² of floor space

9.3

 m^2

75

0

Total

Sub-Total Non-Residential 18000

0

19100

180

L/d

L/d

1452

22952 L/d

22952 L/d

(C) CROZIER						Project No.: Project Name: Revision Date: Prepared By:	Royal Rose 8-Mar-23
	CONSULTING ENGINEERS						Checked By:	
	able 8.2.1.3.A							
	Column 1	Column 2	Per	Unit	Existing Units (Person /	Existing	Proposed	Proposed
	Residential Occupancy	Volume,			Dwelling / m ² / etc)	Flow	Units	Flow
4.	Dwellings	litres			2	(L/dav)	(Person /	(L/dav)
4.	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0
		1100			Sub-Total Residential	1100	L/d	0
BC To	able 8.2.1.3.B							
	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)
2.	Assembly Hall - per seat							
	a) No food service, or	8	1	Seat	0	0		0
	 b) Food service provided 	36	1	Seat	0	0	0	0
12.	Food Service Operations							
	 a) Restaurant (not 24 hour), per seat 	125	1	Seat	0	0	172	21500
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	127	15875
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0
13.	Hospitals - per bed							
	 a) Including laundry facilities, or 	750	1	Bed	0	0	0	0
	b) Excluding laundry facilities	550	1	Bed	0	0	0	0
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0
15.	Office Building ⁽³⁾							
	b) Per each 9.3 m ² of floor space	75	9.3	m ²	0	0	0	0
	•				Sub-Total Non-Residentia	18000	L/d	37375

Total 19100 L/d

37375 L/d

Project No.: 1733-6596 Project Name: Royal Rose Court Revision Date: 08-Mar-23 Prepared By: PM Checked By: NS/GC

ltem	Column 1 Residential Occupancy	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person /	Proposed Flow (L/day)	I
4.	Dwellings						-		
	b) 2 bedroom dwelling	1100	1	Dwelling	1	1100	0	0	
	able 8.2.1.3.B				Sub-Total Residential	1100	L/d	0	L
	Column 1 Establishments	Column 2 Volume, litres	Per	Unit	Existing Units (Person / Dwelling / m ² / etc)	Existing Flow (L/day)	Proposed Units (Person / Dwelling / m2 / etc)	Proposed Flow (L/day)	1
2.	Assembly Hall - per seat								
	a) No food service, or	8	1	Seat	0	0	0	0	
	b) Food service provided	36	1	Seat	0	0	358	12888	
12.	Food Service Operations								
	a) Restaurant (not 24 hour), per seat	125	1	Seat	0	0	172	21500	
	f) Bar and cocktail lounge, per seat	125	1	Seat	0	0	127	15875	
	i) Cafeteria - per meal	12	1	Meal	0	0	0	0	
13.	Hospitals - per bed								
	a) Including laundry facilities, or	750	1	Bed	0	0	0	0	
	b) Excluding laundry facilities	550	1	Bed	0	0	0	0	
14.	Long-Term Care Homes, etc per bed	450	1	Bed	40	18000	0	0	
15.	Office Building ⁽³⁾								
	b) Per each 9.3 m ² of floor space	75	9.3	m ²	0	0	0	0	
	·		•		Sub-Total Non-Residentia	18000	L/d	50263	ן י
					Total	19100	L/d	50263	L

J:\1700\1733-Fusioncorp Dev Inc\6596 - Royal Rose Court\Design\Civil_Water\Water & SAN\2023.03.07_OBC Servicing and SDU Calcs

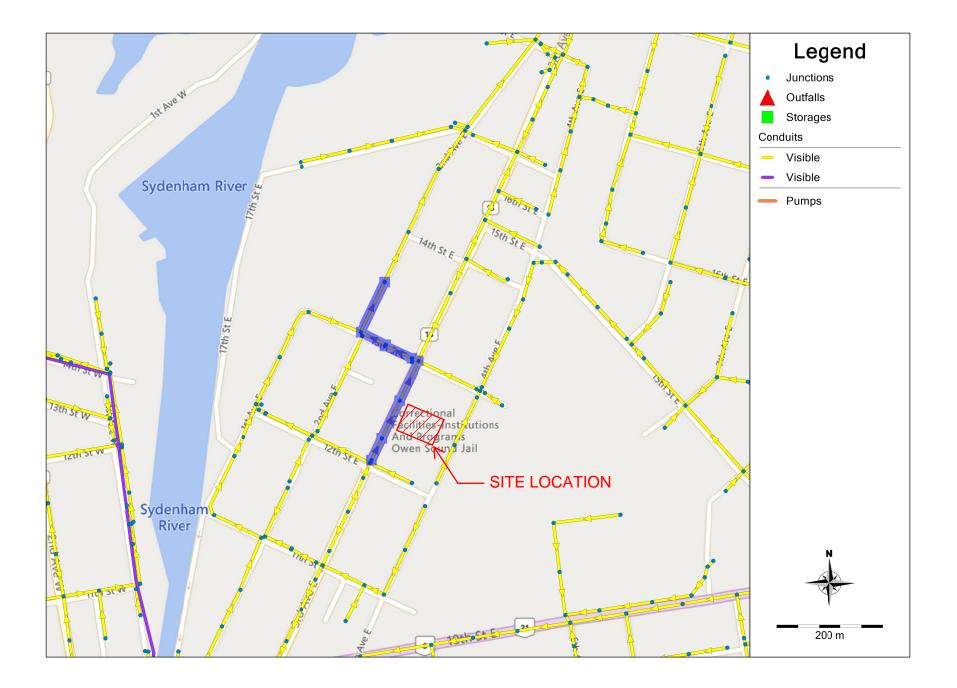


File: 1733-6596 Date: 2023.03.08 By: PM Check By: NS/GC

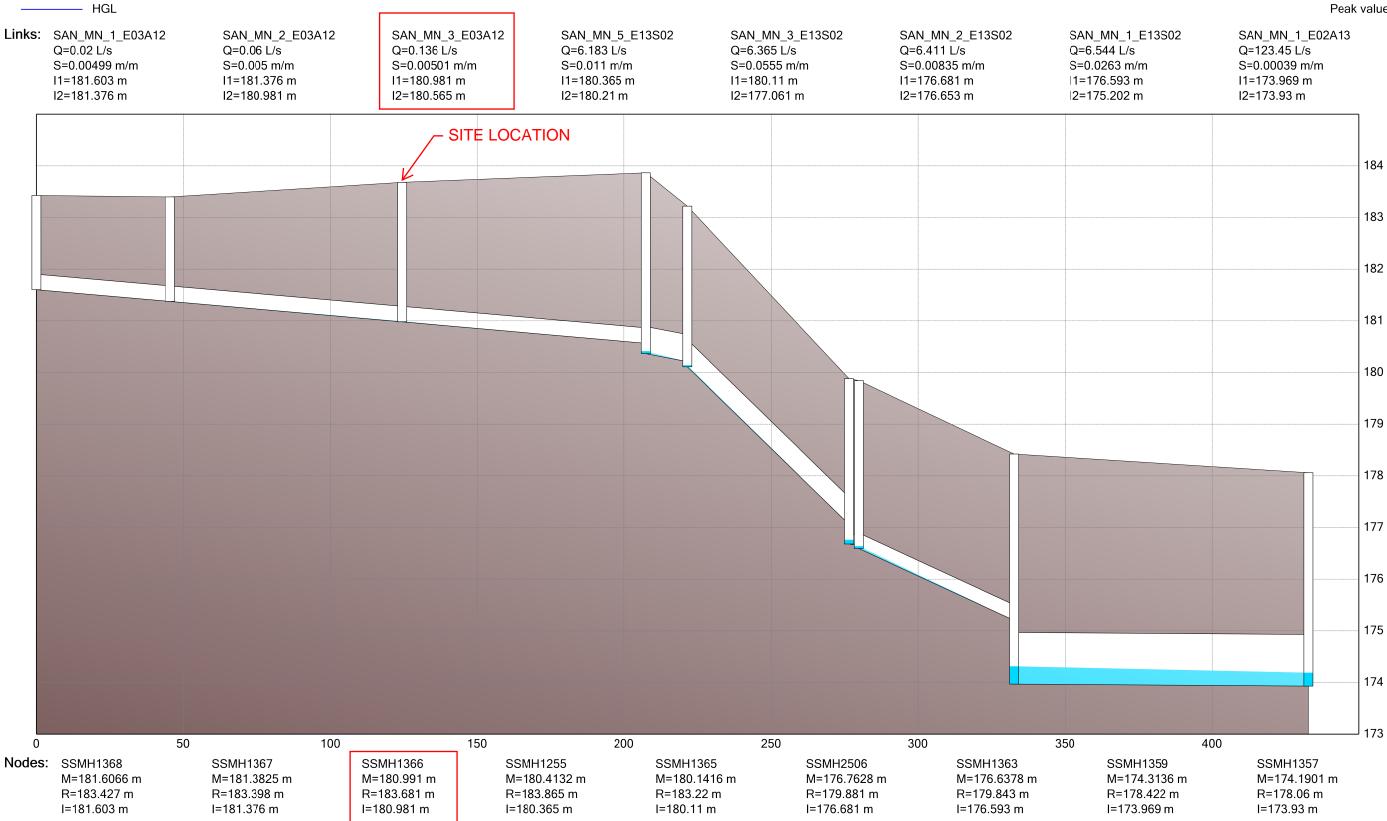
Royal Rose Court - Preliminary Sanitary Design Flow (Existing)

Developed Site Area			0.65	ha
Number of Residential Units Concept Plan (ERS Architects	, 2022.12.09)			
Townhouse		Total Desidential	1	011110
Person Per Residential Unit		Total Residential:		units
Townhouse			2.30	persons/unit
Residential Population			2	persons
Commercial/Institutional Pop	ulation		40	persons
Total Population			42	persons
Unit Sewage flows				
Infiltration			0.20	L/s/ha
<u>Total Design Sewage Flows</u>				
Infiltration/Inflow			0.13	L/sec
Average Daily Residential Flov	W		0.01	L/sec
Average Daily Commercial/Ir	nstitutional Flow		0.21	L/sec
Residential Peak Factor	(Harmon Formula)		4.0	
Institutional Peak Factor	(Harmon Formula)		4.0	
Total Peak Daily Flow			1.01	L/sec

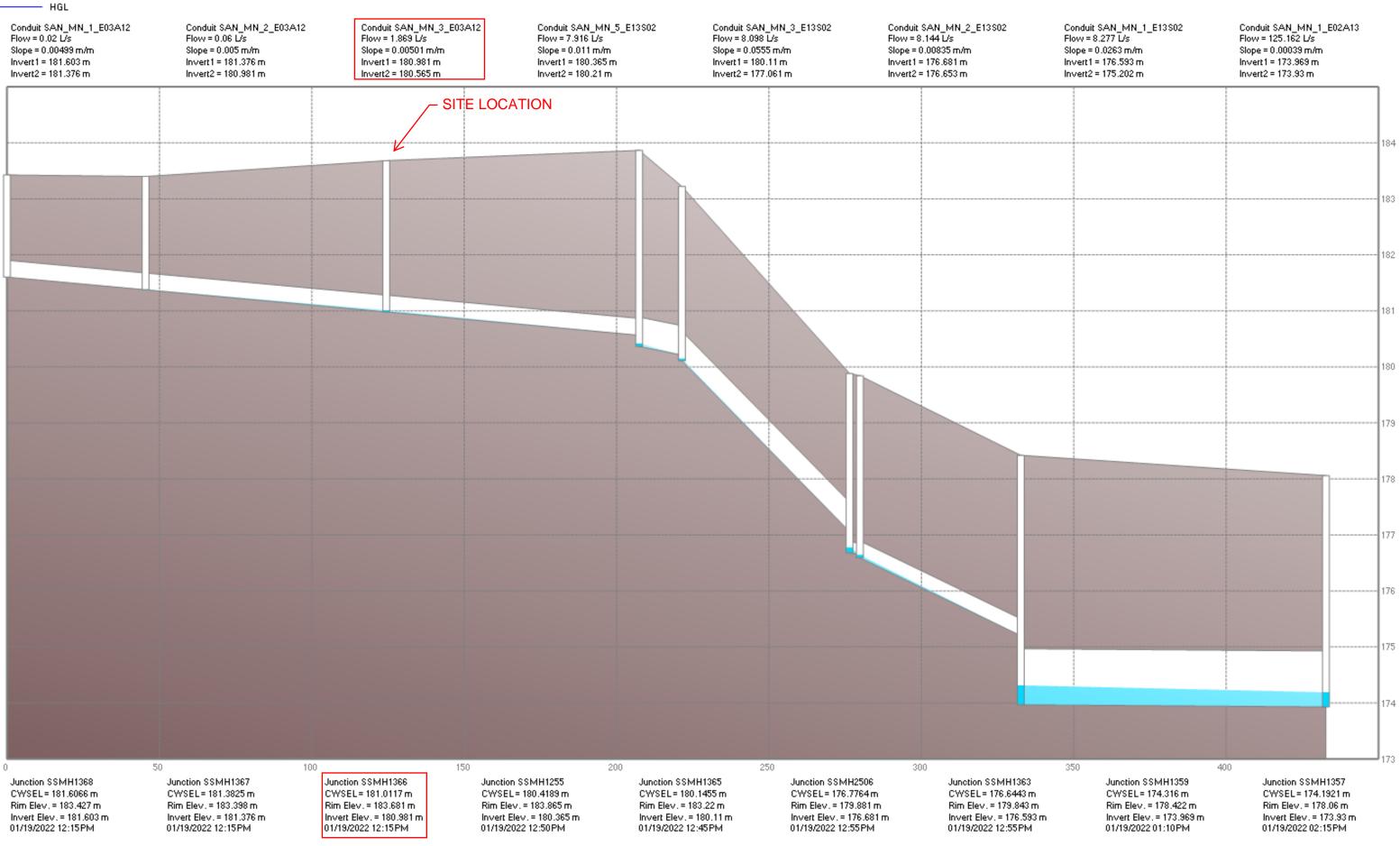
CROZIER & ASSOCIATES Consulting Engineers	File: 1733-6596 Date: 2023.03.08 By: PM Check By: NS/GC								
Royal Rose Court - Preliminary Sanitary Design Flow	Royal Rose Court - Preliminary Sanitary Design Flow (Proposed)								
Developed Site Area	0.65 ha								
<u>Number of Residential Units</u> Commercial/Institutional Population Total Population	657 persons 657 persons								
<u>Unit Sewage flows</u> Infiltration	0.20 L/s/ha								
Total Design Sewage Flows									
Infiltration/Inflow	0.13 L/sec								
Average Daily Commercial/Institutional Flow	0.58 L/sec								
Commercial Peak Factor (Harmon Formula)	3.9								
Total Peak Daily Flow	2.40 L/sec								



EXISTING CONDITIONS - DWF

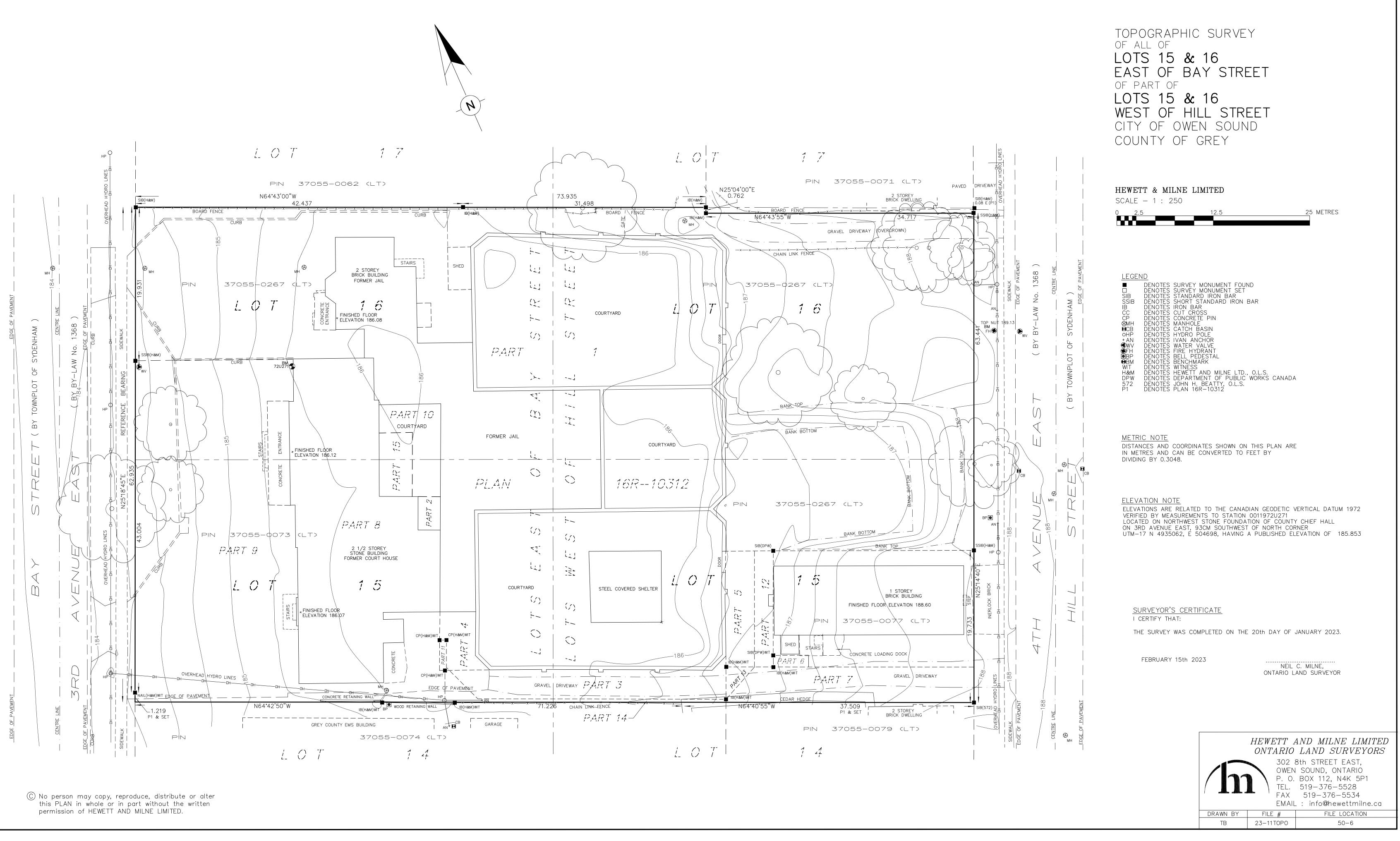


PROPOSED CONDITIONS - DWF



APPENDIX E

Topographic Survey (Hewett Milne Ltd., February 2023)



APPENDIX F

Preliminary Modified Rational Method Calculations



PROJECT: Royal Rose Court PROJECT No.: 1733-6596 DATE: 2023.03.09 DESIGN: PM CHECK: NS/GC

Modified Rational Method

0.12

24.7

Pre-Development Scenario Data								
	Inputs	Outputs	5					
IDF Location	Town of Blue	Mountains	Intensity (mm/hr):	136.52				
Return Period		5 yr						
Time of Concent	tration (min)	5						
Coeff A		1234.576						
Coeff B		8.297						
Coeff C		0.851						
Runoff Coeff (Ur	nadjusted)	0.49	Flow (m ³ /s)	0.12				
Runoff Coeff (Ac	ljusted)	0.49						
Area (ha)		0.64						

Post-Development Scenario Data								
	Inputs	Outputs						
IDF Location	Town of Blue	e Mountains	Intensity (mm/hr):	136.52				
Return Period		5 yr						
Time of Concentra	ation (min)	5						
Coeff A		1234.576						
Coeff B		8.297						
Coeff C		0.851						
Runoff Coeff (una	djusted)	0.70	Uncont. Flow (m ³ /s)	0.17				
Runoff Coeff (Adj	usted)	0.70						
Area (ha)		0.64						

Target Flow (m³/s)

REQUIRED STORAGE VOLUME:

Sto	rage Volume D	eterminatio	n (Detailed)	
T _d	i	Τ _d	Q _{Uncont}	Sd
min	mm/hr	sec	m³/s	m ³
5	136.52	300	0.173	15.6
10	104.05	600	0.132	24.7
15	84.71	900	0.107	24.1
20	71.79	1200	0.091	18.5
25	62.51	1500	0.079	10.0
30	55.49	1800	0.070	0.0
35	49.99	2100	0.063	0.0
40	45.55	2400	0.058	0.0
45	41.89	2700	0.053	0.0
50	38.81	3000	0.049	0.0
55	36.19	3300	0.046	0.0
60	33.92	3600	0.043	0.0
65	31.94	3900	0.040	0.0
70	30.20	4200	0.038	0.0
75	28.65	4500	0.036	0.0
80	27.26	4800	0.035	0.0



PROJECT: Royal Rose Court PROJECT No.: 1733-6596 DATE: 2023.03.09 DESIGN: PM CHECK: NS/GC

Modified Rational Method

0.26

51.3

Pre-Development Scenario Data								
	Inputs	Outputs	5					
IDF Location	Town of Blue	e Mountains	Intensity (mm/hr):	230.33				
Return Period		100 yr						
Time of Concent	tration (min)	5						
Coeff A		2171.754						
Coeff B		8.303						
Coeff C		0.867						
Runoff Coeff (Ur	nadjusted)	0.49	Flow (m ³ /s)	0.26				
Runoff Coeff (Ac	ljusted)	0.61						
Area (ha)		0.64						

Post-Development Scenario Data							
Inputs		Outputs					
IDF Location	Town of Blu	e Mountains	Intensity (mm/hr):	230.33			
Return Period		100 yr					
Time of Concentration (min)		5					
Coeff A		2171.754					
Coeff B		8.303					
Coeff C		0.867					
Runoff Coeff (ur	nadjusted)	0.70	Uncont. Flow (m ³ /s)	0.37			
Runoff Coeff (Ad	djusted)	0.88					
Area (ha)		0.64					

Target Flow (m³/s)

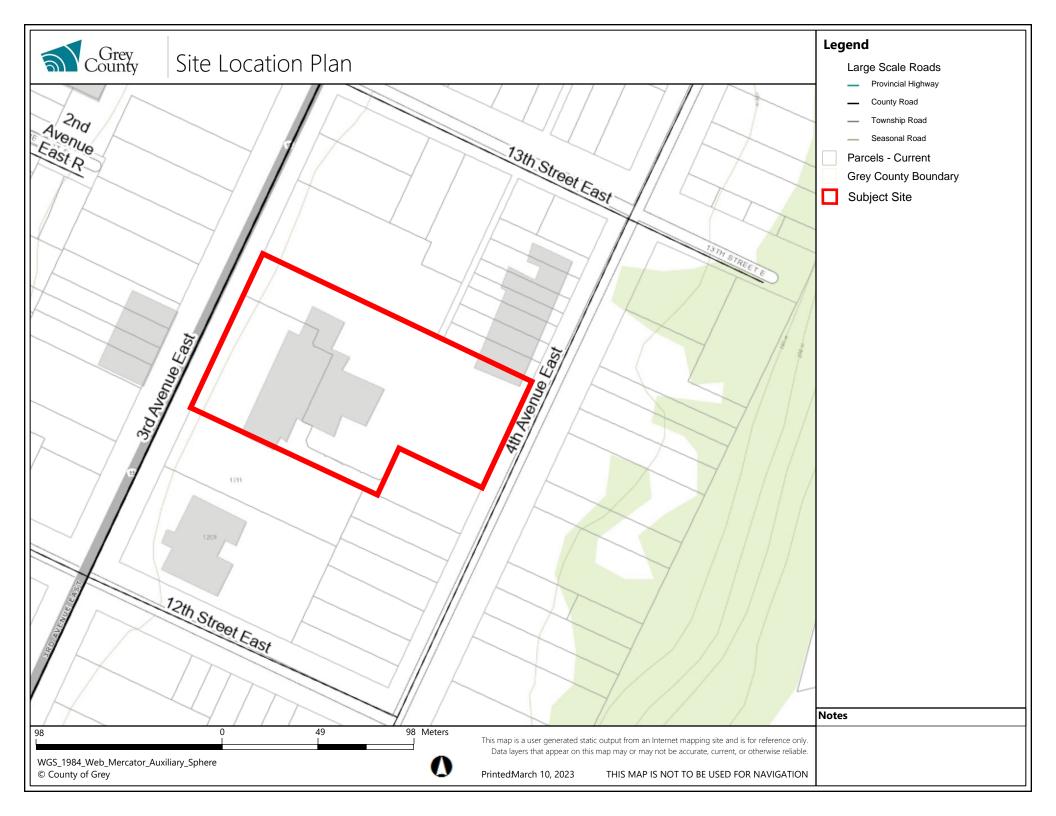
REQUIRED STORAGE VOLUME:

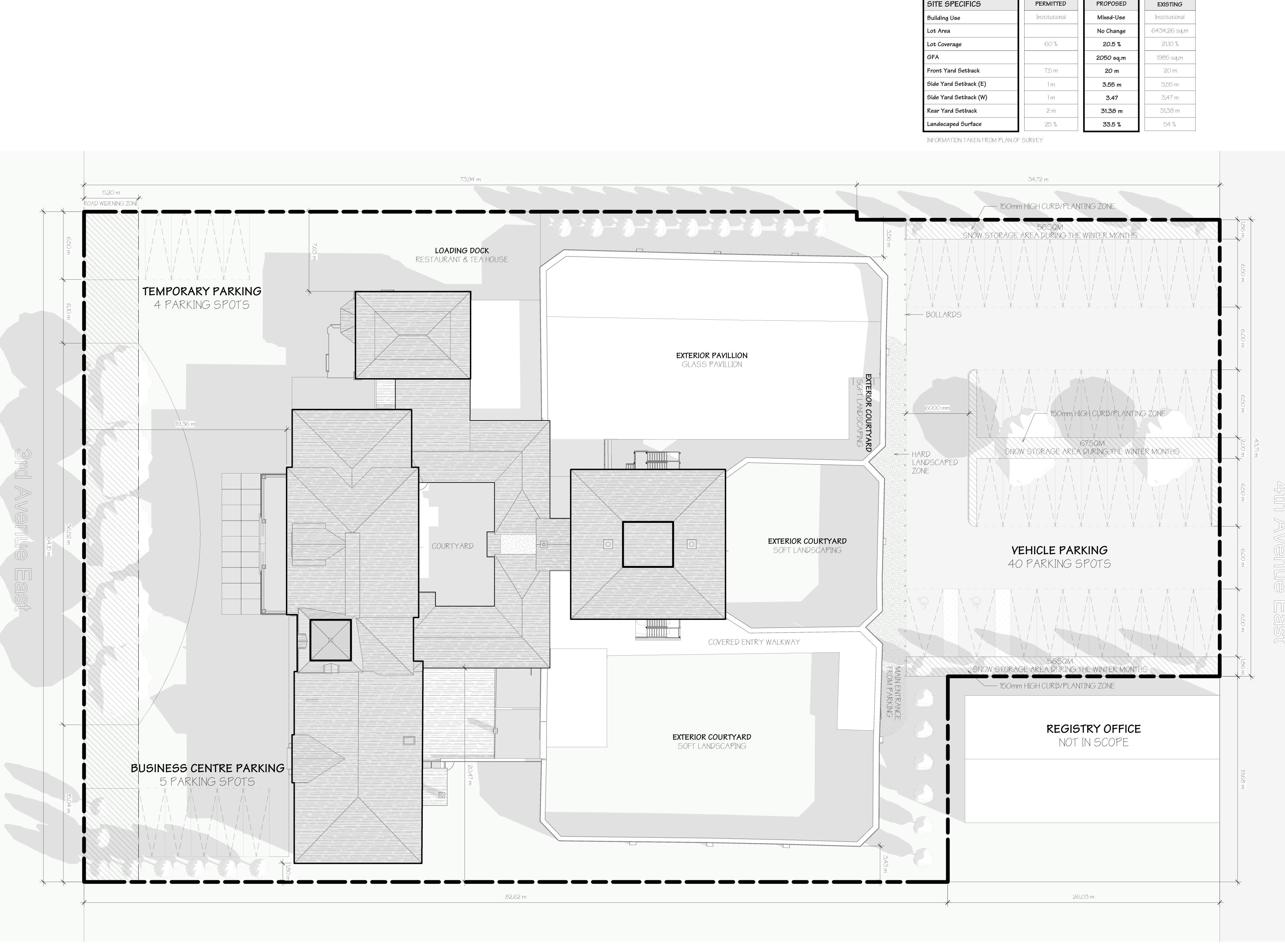
Storage Volume Determination (Detailed)						
Τ _d	i	T _d	Q _{Uncont}	Sd		
min	mm/hr	sec	m³/s	m ³		
5	230.33	300	0.365	33.0		
10	174.66	600	0.277	51.3		
15	141.67	900	0.225	49.0		
20	119.69	1200	0.190	36.4		
25	103.95	1500	0.165	17.6		
30	92.08	1800	0.146	0.0		
35	82.78	2100	0.131	0.0		
40	75.30	2400	0.119	0.0		
45	69.14	2700	0.110	0.0		
50	63.97	3000	0.101	0.0		
55	59.56	3300	0.094	0.0		
60	55.76	3600	0.088	0.0		
65	52.45	3900	0.083	0.0		
70	49.53	4200	0.079	0.0		
75	46.95	4500	0.074	0.0		
80	44.63	4800	0.071	0.0		

FIGURES

Figure 1: Site Location

Figure 2: Site Plan (ERS Architects, December 2022)





OWNERSHIP

ZONING DESIGNATION INSTITUTIONAL

LEGAL DESCRIPTION LOTS 15 & 16 EAST OF BAY ST & LOTS 15 & 16 WEST OF HILL ST PROPERTY ADDRESS 1234-1259 3rd AVENUE EAST, OWEN SOUND, ONTARIO

FC Hospitality & Entertainment Inc.

SITE SPECIFICS	PERMITTED	PROPOSED
Building Use	Institutional	Mixed-Use
Lot Area		No Change
Lot Coverage	60 %	20.5 %
GFA		2050 sq.m
Front Yard Setback	7.5 m	20 m
Side Yard Setback (E)	1 m	3.55 m
Side Yard Setback (W)	1 m	3.47
Rear Yard Setback	2 m	31.38 m
Landscaped Surface	25 %	33.5 %
INFORMATION TAKEN FROM PLAN OF	SURVEY	





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Re	Revision Schedule				
No.	Description	Date			
1	Issued for Pre-Consultation Meeting	2022-10-14			

FC Entertainment & Hospitality Inc. **Royal Rose Court** 1235-1259 3rd Avenue East Owen Sound, ON N4K 2L6

SITE PLAN

